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Department of Water Engineering

Defense Report

Towards obtaining the Master's degree in:

Hydraulics

Subject: .

Study of the drinking water transfer from the Tichy-Haf dam to provinces: Bejaia, Setif and Bourdj Bou Arreridj, Algeria

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Dedication

I dedicate this modest work:

To my father, my source of motivation, who has constantly supported and encouraged me
throughout my academic journey;

To my mother, my reason for living, for her love, attention, moral support, and prayers;

To my dear sister and my dear brother;

To my dear and faithful friends;

To the entire hydraulic engineering class of 2019-2020.

ملخص

الهدف من هذه المذكرة هو تصميم شبكة الامداد لمياه الشرب من سد تيشي- حاف، الواقع بولاية بجاية، الى خمسة عشر (15) بلدية تابعة اداريا للولايات التالية: بجاية وبرج بوعريريج و سطيف . في هذا السياق، قمنا بتقدير عدد السكان المتوقعين بحلول عام 2060، ثم حساب معدل التدفق اليومي الأقصى. بعد ذلك، أجرينا دراسة هيدروليكية لتحديد حجم أنابيب الامداد، سواء بالجاذبية أو بالضخ، ثم اخترنا المضخات لثمانية محطات وأخيرًا، قمنا بتحجيم الخزانات لكل بلدية

كلمات مفتاحية: لأنابيب، الضخ، سد، محطة الضخ، خزان.

Résumé:

L'objectif de ce mémoire est de concevoir un réseau d'approvisionnement en eau potable depuis le barrage de Tichy-Haf, situé dans la wilaya de Béjaïa, vers quinze (15) communes relevant administrativement des wilayas de Béjaïa et de Bordj Bou Arréridj et Sétif. Dans ce contexte, nous avons estimé la population prévue d'ici à 2060, puis calculé le débit quotidien maximum. Ensuite, nous avons réalisé une étude hydraulique pour déterminer les dimensions des conduites d'approvisionnement, qu'elles soient par gravité ou par pompage, et avons choisi Epanet pour les pompes de huit stations. Enfin, nous avons dimensionné les réservoirs pour chaque commune.

Mots-clés: conduites, pompage, station de pompage, réservoir, coup de bélier, barrage.

Abstract:

The aim of this thesis is to design a drinking water supply network from the Tichy-Haf dam, located in the province of Béjaïa, to fifteen (15) municipalities under the administrative jurisdiction of the provinces of Béjaïa and Bordj Bou Arréridj and Setif. In this context, we estimated the expected population by 2060 and calculated the maximum daily flow rate which equal 60000 m³/day. Subsequently, we conducted a hydraulic study to determine the pipe sizes for both gravity and pumped systems, selecting for pumps at eight stations. Finally, we sized the reservoirs for each municipality.

Keywords: pipes, pumping, pumping station, reservoir, water hammer, dam.

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List of abbreviations

D: Diameter (m).

ND: Nominal Diameter (m).

NP: Nominal pressure (bar).

ΔH : Head loss in (m).

NGR_{UP} : Upstream Natural ground level in (m).

NGR_{DW} : Downstream Natural ground level in (m).

λ : Darcy friction coefficient.

g: acceleration due to gravity (9.81 m/s^2).

D_{inn} is pipe inner diameter (m).

Hg: geometric head (m).

$NPSH_d$: Net Positive Suction Head (in, meter water column 'mwc').

$NPSH_r$: Net Positive Suction Head (in mwc).

P_{atm} : atmospheric pressure in meters of head (mwc).

B: Maximum value of water hammer (m).

a: Wave velocity (m/s).

TDH: Total Dynamic Head (m).

CMC: Charging chambre (Chambre d Mise en Charge).

PS: Pumped Station.



Introduction

Introduction

Introduction

Hydraulics studies the movements and characteristics of water, closely linked to the transfer of drinking water by conveyance, which transports this resource from sources to communities. Among these sources are dams. The transfer from dams to communities is crucial to ensure a safe and stable water supply. Dams store freshwater and regulate it to maintain a constant supply. Once captured, water is conveyed to urban and rural centers via sophisticated networks, ensuring continuous access to this vital resource.

Unfortunately, there are still areas or municipalities suffering from water scarcity nowadays. This situation is often attributed to rapid population growth in many regions. In Algeria, some areas are particularly affected, such as the provinces of Bordj Bou Arreridj, Sétif, and Béjaïa, where several municipalities face difficulties related to water supply.

As part of my professional master's program at the Institute of Applied Sciences and Techniques (ISTA-Bouira), I completed a four-month internship at AMENHYD company. During this internship, I have chosen to conduct a study aiming to transport water from the Tichy-Haf dam (Bejaia province) to the provinces of Béjaïa and Bordj Bou Arreridj, Setif.

This study focuses on the following parts to achieve the given objective:

- ✓ Internship Host Company;
- ✓ Project presentation and study area;
- ✓ Assessment of water needs;
- ✓ Study of the water supply network;
- ✓ Selection of pumps for pumping stations;
- ✓ Sizing of reservoirs;

This work was finally closed by a conclusion.

Chapter I

Internship Host

Company

I.1 Introduction

At the end of each training cycle at the Institute of Technology (IT. Bouira), each student must complete a final project simultaneously with an internship that lasts approximately four months. The purpose of this project is to:

- Enhance the knowledge acquired during the training cycles;
- Integrate into the world of work (the professional environment);
- Understanding the duties and responsibilities of a hydraulic engineer.

As a result, I am required to exert possible effort, make observations, and remarks in order to present this work. For this reason, I had chosen **AMENHYD** company to pass the internship and carry out my final study project.

I.2 History of AMENHYD company

AMENHYD a contraction of the words "**Aménagement, Environnement, Hydraulique**" is a leader in environmental-related professions for 25 years. It is a private Algerian company that designs solutions for water, environment, construction, deconstruction, and engineering.

The company began its activities in the field of construction and public works in 1994 under the name "**ETB/TCE Chelghoum Djamel Eddine**". The AMENHYD group was founded in 2003 to meet the needs of the Algerian market [01].

I.3 Presentation of the AMENHYD company

AMENHYD company includes 2300 employees, 3030 physical units, 12 billion DZD average business and 3.654 billion DZD share capital.

General information about AMENHYD company

Nature: Secondary establishment (Agency)

Year of establishment: 2003

Legal form: SPA

Capital: 3.654.724.000 DZD (Algerian Dinar)

Registration No: 03B0021465

Company staff: 1.000 to 4.999 employees

Kompass ID: DZ019124

Head Office: Lotissement Boushaki F. Lot No. 28 BP 21. Cité Djorf 1 6024 Bab Ezzouar

Number phone: +213 23 83 21 20

Fax: +213 23 83 29 28

Email: <http://www.amenhyd.com>



Figure I.1: Logo of AMENHYD company

I.3.1 The AMENHYD Vision

To be the most efficient and significant company in environmental protection in Africa by 2030 [01].

I.3.2 The mission of AMENHYD

To enhance water resources and protect the environment through reliable, Innovative, and digital solutions, facilitated by [01]:

- A high-performing team;
- Strong partnerships;
- Optimal processes.

I.3.3 The subsidiaries of the AMENHYD

The subsidiaries of the AMENHYD group are primarily composed of the following [01]:

AMENHYD 'HYDRAULIC ENVIRONMENTAL DEVELOPMENT': Specializes in water, waste, and civil and industrial construction and demolition.

ALCAHYD 'ALGERIAN HYDRAULIC PIPEWORK': Prefabrication of reinforced concrete elements and hydraulic pipelines.

GATECH Electrical engineering automation control.

International Process: Manufactures process and mechanical equipment.

BIMECA: An engineering office dedicated to mechanical equipment studies.

I.3.4 The services and activities of AMENHYD

- ✓ System design studies;
- ✓ Installation and commissioning services;
- ✓ Repair and maintenance of facilities;
- ✓ Rehabilitation of stations;
- ✓ Operation of treatment plants;
- ✓ Training and technical assistance;
- ✓ Monitoring and supervision.

I.4 The geographic location of AMENHYD

It is located in the Rabia Tahar neighborhood of the Bab Ezzouar municipality, in Algiers.

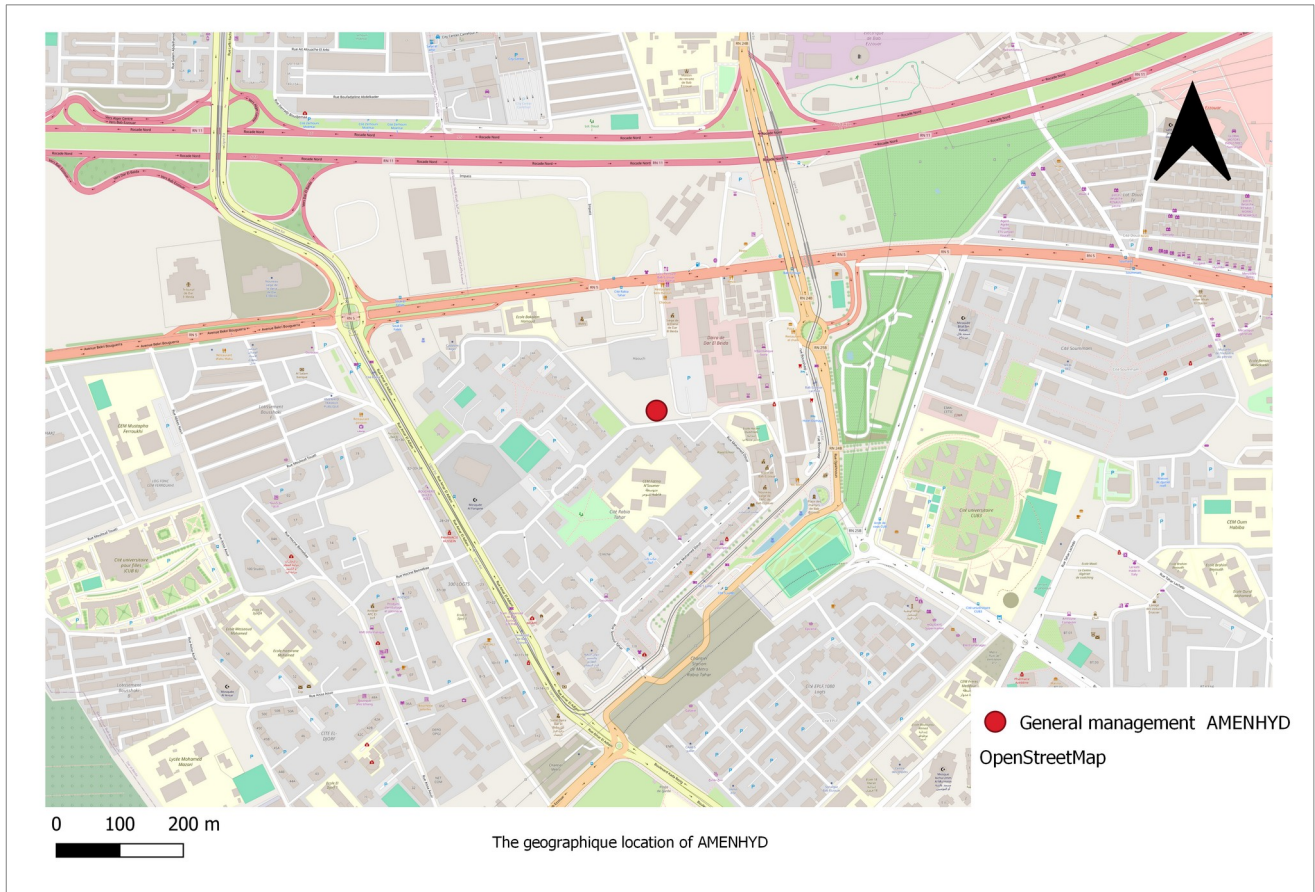
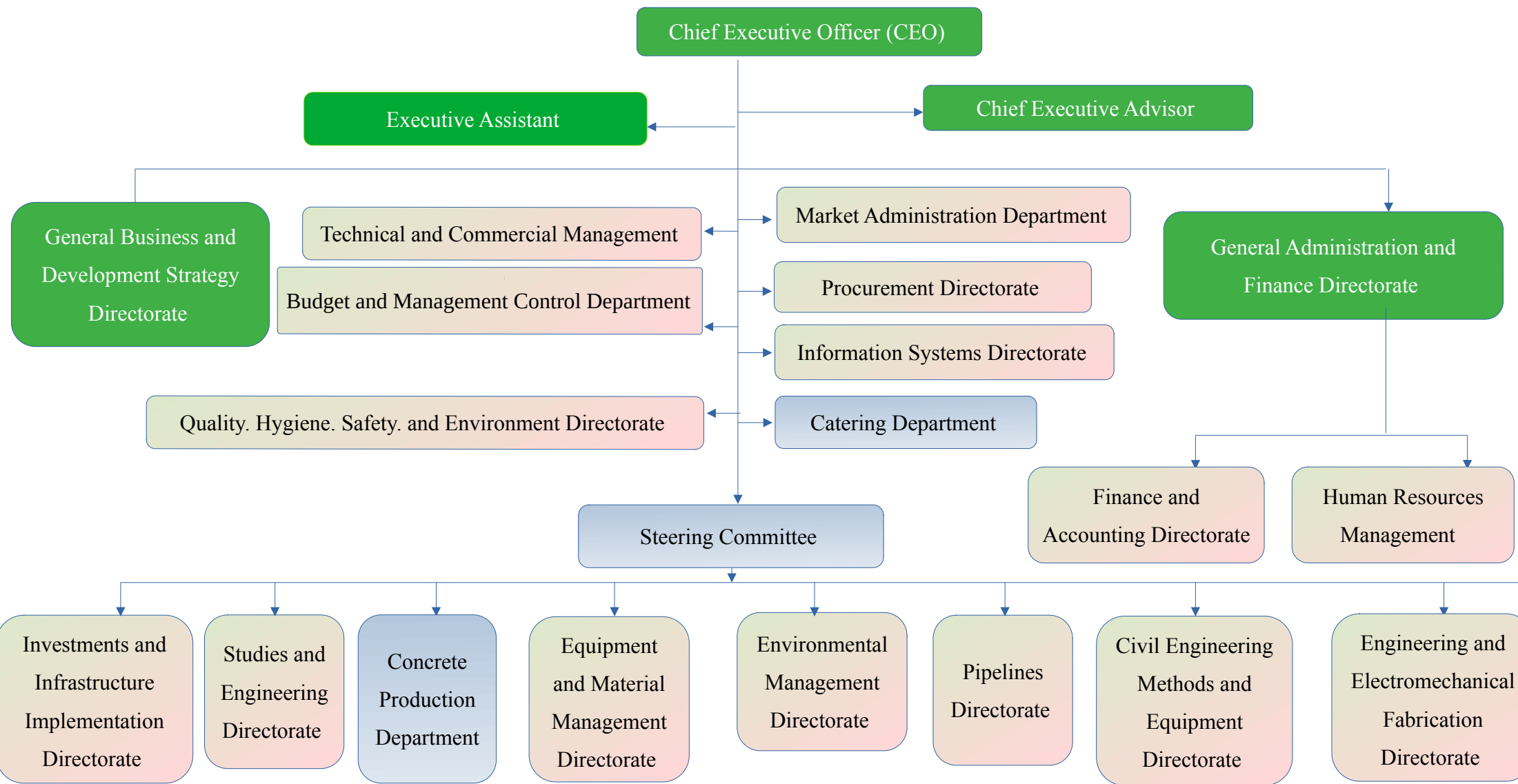
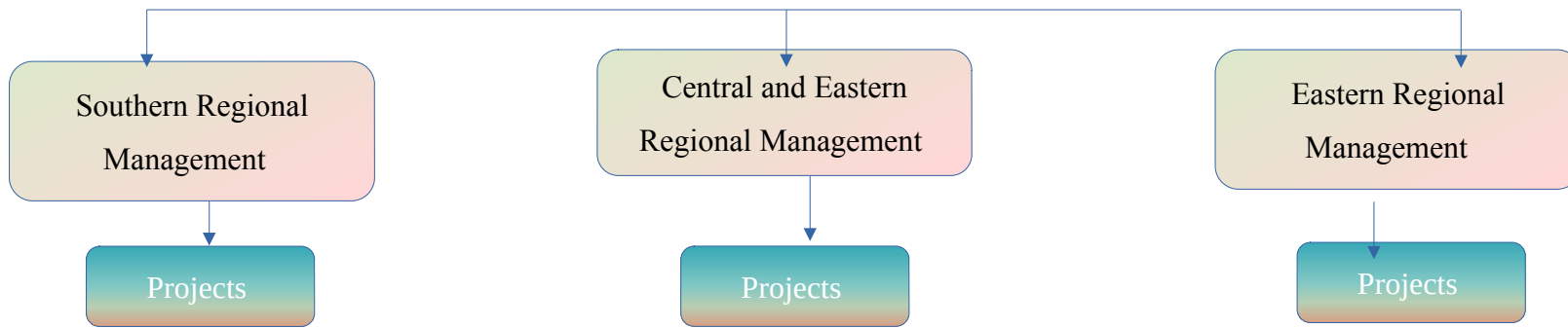


Figure I.2: The geographic location of AMENHYD company

I.5 The organizational chart of the company

The organization of AMENHYD-SPA is summarized in the following organizational chart [01]:





Drawing I.1: Organizational chart of the company AMENHYD

I.6 Some projects carried out by the company

Water treatment[01]:

- Berriane Treatment Plant (Ghardaïa Province), in 2013;
- Guerrara Treatment Plant (Ghardaïa Province), in 2014.

Demineralization plants [01]:

- Touggourt Treatment Plant (Ouargla Province) (production capacity of 34.000 m³/day);
- Meghayer Treatment Plant (El Oued Province) (production capacity of 12.000 m³/day).

Wastewater treatment plants (WWTP) by lagoon treatment [01]:

- Tamanrasset WWTP, Ghardaïa WWTP, Beni Messous WWTP (Algiers Province);

Sanitation [01]:

- Development works of Oued Ouchayah (Algiers Province), in 2016;
- Beni Messous Monolith Collector (Algiers Province), in 2015;
- Sanitation of the Mzab Valley (Ghardaïa), in 2012.

I.7 Conclusion

In conclusion, this chapter was conducted to discover the key aspects of the company AMENHYD. We have learned about its history, structure, objectives, and the values it upholds. Additionally, we have also explored professional skills such as teamwork, time management, and communication, which are important in this context.

In the next chapter, we will discuss the project presentation and the study area of this project.

Chapter II

Study Area and Project

Presentation |

II.1 Introduction

Before implementing this project, studying the study area is necessary. This process enables us to understand the various factors that influence its design. In this chapter, we will focus on presenting the study area and its characteristics, as well as presenting the studied project.

II.2 The project's objective

The objective of the project is providing potable water from the Tichy-Haf dam to the study area covering a total of 14 municipalities, distributed between Setif and Bordj Bou Arreridj provinces, as well as one municipality from the wilaya of Bejaïa, for the horizon of 2060.

II.3 Project description

Providing potable water from the Tichy-Haf dam, with a capacity of 80 million cubic meters, to a study area encompassing a total of 14 municipalities distributed between the provinces of Setif and Bordj Bou Arreridj. These municipalities are situated at the western extremities of Setif province and the northwest of Bordj Bou Arreridj province, along with one municipality from Bejaïa province. This project is currently under construction.

- ✓ In Setif province: Beni Ourtilene, Beni Chebana, Ain Lagredj, Beni Mohli, Guenzet, and Harbil;
- ✓ In Bordj Bou Arreridj province: Djafraa, Teffreg, Colla, El Main, Tassamert, Ouled Dahmane, Bordj Zemmoura, and Theniet Enaser;
- ✓ In Bejaïa province: Bouhamza.

The project will be divided into three lots:

- ➔ **Lot number 01:** from Tichy-Haf dam to the new treatment station with a capacity of 60000 m³/day (located near the Tichy-Haf dam);

- ➔ **Lot number 02:** includes 5 municipalities (Djafraa, Teffreg, Colla, El Main, Theniet Enaser), and the municipality of Bouhamza (Bejaia province) in addition. The later municipality was added to the project in 2024;
- ➔ **Lot number 03:** contains 9 municipalities, which are: Beni Ourtilene, Beni Chebana, Ain Lagredj, Beni Mohli, Guenzet, and Harbil, Tassamert, Ouled Dahmane, Bordj Zemmoura.

Remark:

The part of the transfer project concerned by the present study will be limited only to the lot number 2, which includes the following municipalities from the province of Bordj Bou Arreridj: Djafraa, Teffreg, Colla, El Main, and Theniet Enaser, as well as the municipality of Bouhamza from the province of Bejaïa.

II.4 Tichy-Haf dam

The Tichy-Haf dam, located in the village of Mahfouda, Bouhamza municipality, in the province of Bejaïa, is a gravity-arch dam standing at a height of 90 meters and with a capacity of 80 million cubic meters. It is fed by the Bousellam river. This dam primarily serves for irrigation purposes in the agricultural areas of Sahel and Hight Soummam, as well as for supplying drinking and industrial water to the municipalities along the Akbou-Bejaïa corridor, in addition to flood control of the Bousellam river [02].



Figure II.1: Tichi Haf Dam [02]

II.4.1 The geographical location of the Tichy-Haf dam

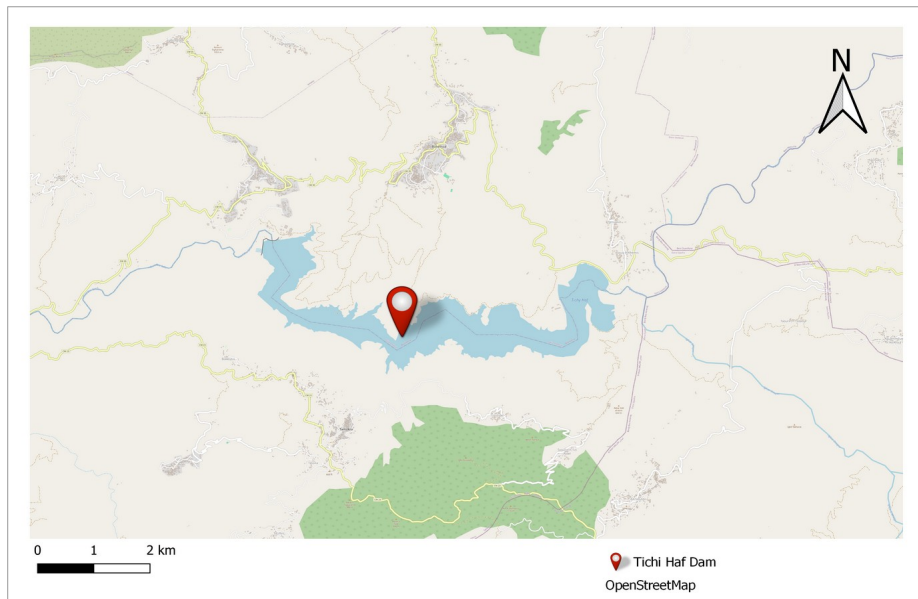


Figure II.2: Geographical location of the Tichy-Haf dam

II.5 Geographical location of the study area

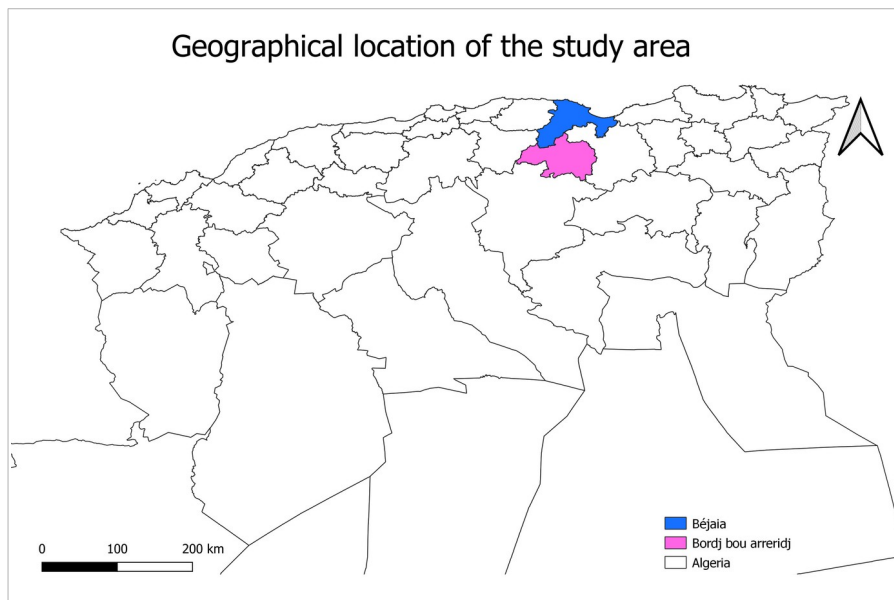


Figure II.3: Geographical location of the study area

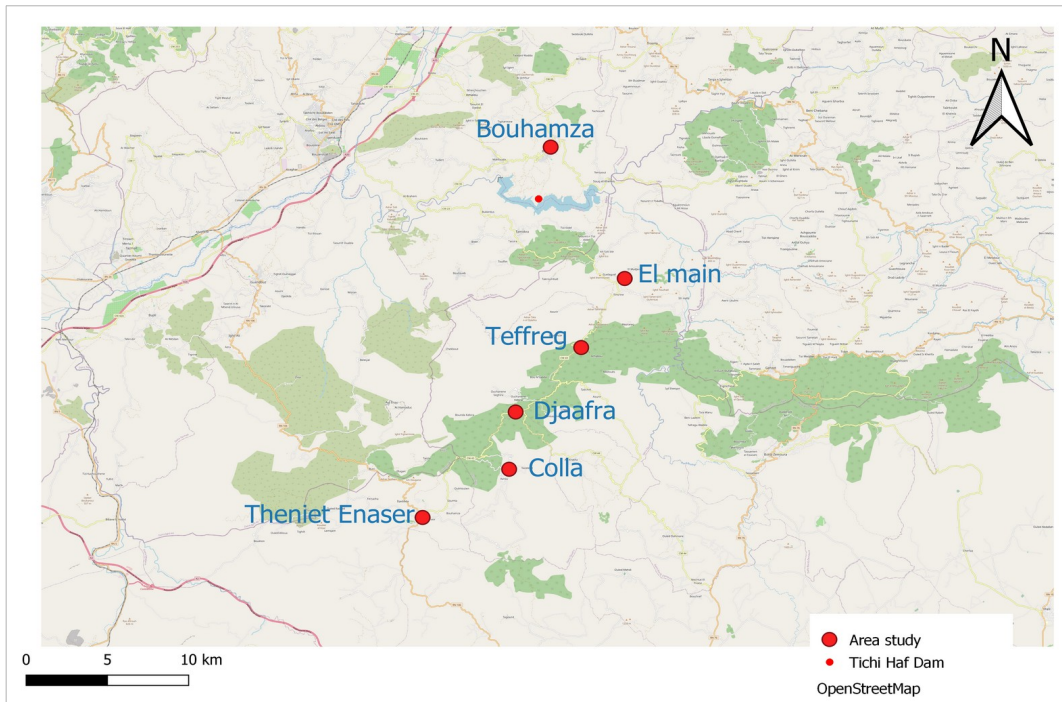


Figure II.4: Geographical location of the study area

II.6 General description of the study area

II.6.1 Climate

The climate in the study area varies; in the north, it is semi-Mediterranean, while in the southwest, it is semi-arid. Summers are hot and dry, and winters are extremely cold. Rainfall is scarce and irregular in both time and location; in the Babor Mountains, precipitation averages 700 mm/year, while in the southwest, it does not exceed 300 mm. Snowfall occurs in winter in all mountains with altitudes exceeding 800 meters [02].

II.6.2 Temperature and humidity

The temperatures are low in winter and high in summer. Moreover, the mediocre atmospheric humidity further extends the dry season [02].

II.6.3 Winds

The winds are influenced by the mountainous topography and regional meteorological conditions. Typically, the main winds come from the west and northwest, but they can change in strength and direction depending on the season. During winter, cold and wet winds may come from the north, bringing rain or snow, especially in the mountains. In summer, hot and dry winds from the south are common, leading to hot and dry weather [02].

II.6.4 Vegetation

The vegetation in the region of the Oued Bousellam its tributaries in the Kabylie of Akbou is diverse due to the various climates and terrains. Forests, meadows, and cultivated areas can be found there.

Forests are mainly composed of oaks, pines, and cedars, which are abundant in mountainous areas and provide important habitats for many animal and plant species.

Meadows and cultivated lands are typically located in valleys and alluvial plains, where agriculture is practiced to grow cereals, olives, fruits, and vegetables [02].

II.7 Conclusion

In this chapter, a general overview of the project that we will be studying was provided. Additionally, the study area was presented, explaining its geographical location and characteristics.

The purpose of this study is to provide potable water from the Tichy-Haf dam to the new treatment station, and then to Lot Number 02, which concerns six (06) municipalities.

In the next chapter, assessment of water needs for all municipalities, projected up to the year 2060, will be discussed.

Chapter III

Assessment of water needs

III.1 Introduction

A potable water supply network is a series of infrastructures designed to provide drinking water to a community to meet human, industrial, and public needs. Before beginning the design of pipelines and others structures, it is essential to assess the needed water flow rates.

In this chapter, we will evaluate population and water needs of each municipality in the three provinces (Bejaia, Bordj Bou Arreridj, Setif) for the future horizon (2060).

III.2 Estimation of water needs

Distribution system must meet the needs (public, domestic, industrial) of its users. But in this study, detailed data about public and industrial needs for the considered horizon (2060) are not available. Therefore, only domestic water needs are evaluated, while the water needs for the other categories are obtained from the AMENHYD company.

III.2.1 Population Assessment

Determining domestic water needs requires estimating the population within the study horizon n . This number of inhabitants can be calculated for future horizons using the compound interest law provided by the following formula [03]:

$$P_n = P_0(1 + \tau)^n \quad (\text{III.1})$$

with:

- P_n : Population within the study horizon n ;
- P_0 : Current population (inhabitants);
- n : Number of years between the considered horizon;
- τ : Average annual population growth rate. For more details, refer to annex number 01.

The results of calculation are presented in the following table:

Table III.1: Population evolution and projection

Province	Municipality	Agglomerated population 2008	Annual Population Change (%)	Agglomerated population (projection 2020)	Agglomerated population (projection 2024)	Agglomerated population (projection 2030)	Agglomerated population (projection 2040)	Agglomerated population (projection 2050)	Agglomerated population (projection 2054)	Agglomerated population (projection 2060)
Bordj Bou-Arredj	Colla	6123	1.3	7150	7529	8135	9257	10533	11092	11985
	Djaafra	7998	1.3	9339	9834	10626	12092	13759	14488	15656
	El main	6237	1.3	7283	7669	8287	9429	10729	11298	12209
	Taffreg	2164	1.3	2527	2660	2875	3272	3723	3920	4236
	Ouled Dehmane	16501	1.3	19267	20289	21924	24947	28386	29891	322100
	Thassameurt	4134	1.3	4827	5083	5493	6250	7112	7489	8092
	Bordj Zemmoura	10296	1.3	12022	12660	13680	15566	17712	18651	20154
	Theniet Annesr	5775	1.3	6743	7101	7673	8731	9935	10461	11304
Setif	Ain Legredj	14668	1.3	17127	18035	19489	22175	25233	26571	28712
	Béni Chévana	13174	1.3	153823	16198	17504	19916	22662	23864	25787
	Béni Mohli	8521	1.3	9949	10477	11321	12882	14658	15436	16679
	Béni Ourtilane	10591	1.3	12366	13022	14072	16011	18219	19185	20731
	Harbil	3675	1.3	4291	4519	4883	5555	6322	6657	7194
	Guenzet	3541	1.3	4135	4354	4703	5353	6091	6415	6931
Bejaia	Bouhamza	9123	0.64	9849	10103	10498	11189	11926	12235	12712

Table III.2: Sparse population (projection)

Province	Municipality	Sparse population projection 2050	Annual Population Change (%)	Sparse population projection 2054	Sparse population projection 2060
Bordj Bou-Argeridj	Colla	715	1.3	753	814
	Djaafra	1535	1.3	1616	1747
	El main	796	1.3	838	906
	Taffreg	498	1.3	524	567
	Ouled Dehmane	2908	1.3	30622	3309
	Thassameurt	1021	1.3	1075	1162
	Bordj Zemmoura	1552	1.3	1634	1766
	Theniet Annesr	3158	1.3	3325	3593
Setif	Ain Legredj	2187	1.3	2303	2489
	Béni Chébana	2067	1.3	2177	2352
	Béni Mohli	976	1.3	1028	1111
	Béni Ourtilane	890	1.3	937	1013
	Harbil	725	1.3	7634	825
	Guenzet	1049	1.3	1105	1194
Bejaia	Bouhamza	0	0.64	0	0

Water endowment

The estimation of water needs for an urban area requires us to provide a fixed standard for each category of consumer (allocation)[03]:

Table III.3: Average unit endowment

Population	Endowment (l/day/inhabitant)
Agglomerated population	150
Sparse areas	95

III.2.2 Consumption flow rates

→ **Average daily flow rate:** given by the formula:

$$Q_{ave.day} = P * D \quad (III.2)$$

Where:

- $Q_{ave.day}$: Average daily flow rate in(l/day);
- P : population (inhabitants);
- D : endowment (l/day/inhabitant).

→ **Maximum daily flow rate:** given by the formula:

$$Q_{max.day} = \frac{Q_{ave.day} \times K_{max.day}}{1000} \quad (III.3)$$

Where.

- $Q_{max.day}$: Maximum daily flow rate in(m³/day);
- $K_{max.day}$: Maximum daily irregularity coefficient between (1.3-1.6).

Remark:

The following table shows the water requirements for the 2060 horizon, exclusively. The results for the other horizons are given in annex number 02.

Table III.4: Domestic needs for the 15 municipalities

<i>Province</i>	<i>Municipality</i>	<i>Agglomerated population (projection)</i>	<i>Allocation (l/d/hab)</i>	<i>Qave day (l/d)</i>	<i>K max/d</i>	<i>Qmax/day (l/d)</i>	<i>Sparse population projection 2060</i>	<i>Allocation (l/d/inhab)</i>	<i>Qave day (l/d)</i>	<i>K max /d</i>	<i>Qmax day (l/d)</i>	<i>domestic need (l/d)</i>
Bordj Bou-Arredj	Colla	11986	150	1 797 821.94	1.45	2 606 841.82	814	95	77 290.14	1.45	112 070.70	2 718 912.52
	Djaafra	15656	150	2 348 355.37	1.45	3 405 115.28	1 747	95	165 930.58	1.45	240 599.35	3 645 714.63
	El main	12209	150	1 831 294.38	1.45	2 655 376.85	906	95	86 046.09	1.45	124 766.83	2 780 143.67
	Taffreg	4236	150	635 388.97	1.45	921314.01	567	95	53 832.85	1.45	78 057.64	999 371.65
	Ouled Dehmane	322100	150	4 844 987.74	1.45	7 025 232.22	3 309	95	314 349.27	1.45	455 806.45	7 481 038.66
	Thassameurt	8092	150	1 213 816.09	1.45	1 760 033.33	1 162	95	110 368.16	1.45	160 033.83	1 920 067.16
	Bordj Zemmoura	20153	150	3 023 089.13	1.6	4 836 942.61	1 766	95	167 768.25	1.6	268 429.20	5 105 371.81
	Theniet Annesr	11304	150	1 695 642.94	1.45	2 458 682.27	3 593	95	341 373.80	1.45	494 992.01	2 953 674.27
Setif	Ain Legredj	28712	150	4 306 786.26	1.45	6 244 840.08	2 489	95	236 410.54	1.45	342 795.29	6 587 635.37
	Beni Chébana	25788	150	3 868 121.23	1.45	5 608 775.79	2 352	95	223 438.77	1.45	323 986.22	5 932 762.00

	Beni Mohli	16679	150	1 667 944.99	1.45	2 418 520.24	1 111	95	105 503.75	1.45	152 980.43	2 571 500.67	
	Beni Ourtilane	20731	150	3 109 706.39	1.45	4 509 074.26	1 013	95	96 207.31	1.45	139 500.60	4 648 574.86	
	Harbil	7194	150	1 079 045.51	1.45	1 564 615.99	825	95	78 371.12	1.45	113 638.13	1 678 254.11	
	Guenzet	6931	150	1 039 700.72	1.45	1 507 566.04	1 194	95	113 394.91	1.45	164 422.60	1 671 988.66	
Bejaia	Bouhamza	12712	150	1 906 784.42	1.45	2 764 837.41	0.00	95	0.00	1.45	0.00	2 764 837.41	
					Total	50 287 768.19 l/day					Total	3 172 079.28 l/day	53 459 847.47 l/day
											Total	53 459.85 m ³ /day	

Interpretation:

In this table, we observe that the projected population for the year 2060 ranges between 4000 and 32000, inhabitants while for sparse population, it ranges from 800 to 3000 inhabitants. Regarding the maximum daily irregularity coefficient (k), we have opted for 1.45 in normal cities and 1.6 for tourist cities. Finally, we conclude that:

- ✓ For agglomerated population: $Q_{max.day} = 50287768.19$ l/day;
- ✓ For sparse population: $Q_{max.day} = 3172079.28$ l/day;
- ✓ So domestic needs for the 15 municipalities equals 53459.85 m³/day.

Table III.5: The total water requirements for the 15 municipalities

Horizon of 2060				Water needs m ³ /day				
Province	Municipality	Agglomerated population (projection)	Sparse population projection 2060	Domestic need (m ³ /day)	Public sector (m ³ /day)	Industrial sector (m ³ /day)	Tertiary sector (m ³ /day)	TOTAL
Bordj Bou-Arreidj	Colla	10 533	715	2 718.91	215.92	41.52	83.05	3 059.40
	Djaafra	13 759	15087	3 645.71	283.73	54.56	109.13	4 093.13
	El main	10 729	11935	2 780.14	219.29	42.17	84.34	3 125.94
	Taffreg	3 723	4108	999.37	70.24	13.51	27.01	1 110.13
	Ouled Dehmane	28 386	30739	7 481.04	575.38	110.65	221.3	8 388.37
	Thassameurt	7 112	7840	1 920.07	134.91	25.94	51.89	2 132.81
	Bordj Zemmoura	17 712	18204	5 105.37	338.65	65.12	130.25	5 639.39
	Theniet Annesr	9 935	7903	2 953.67	162.29	31.21	62.42	3 209.59
Setif	Ain Legredj	25 233	29137	6 587.64	538.36	103.53	207.06	7 436.59
	Beni Chébana	22 663	25421	5 932.76	471.67	90.71	181.41	6 676.55
	Beni Mohli	14 658	16240	2 571.50	297.07	57.13	114.26	3 039.96
	Beni Ourtilane	18 219	22144	4 648.57	399.62	76.85	153.7	5 278.74
	Harbil	6 322	7157	1 678.25	120.6	23.19	46.39	1 868.43

	Guenzet	6 092	6264	1 671.99	110.67	21.28	42.57	1 846.51
Bejaia	Bouhamza	11 926	0	2 764.84	0	0	0	2 764.84
TOTAL		207 002	202894	53 459.85	3938.4	757.37	1514.78	59 670.40 m ³ /day

Interpretation:

In this table, it is evident that new data has been incorporated, including water requirements for the public, industrial, and tertiary sectors. So, the results are as follows:

- ✓ Public sector: $Q_{max.day} = 3938.4 \text{ m}^3/\text{day}$;
- ✓ Industrial sector: $Q_{max.day} = 757.37 \text{ m}^3/\text{day}$;
- ✓ Tertiary sector: $Q_{max.day} = 1514.78 \text{ m}^3/\text{day}$;

Consequently, the total sum amounts to $59670.40 \text{ m}^3/\text{day}$.

III.3 Conclusion

In this chapter, both agglomerated population and the sparse population were assessed for each municipality projected for the year 2060. Additionally, consumption flow rates were evaluated, including average daily flow rate and maximum daily flow rate. It's crucial to incorporate the values provided for the public, industrial, and tertiary sectors obtained from the AMENHYD company. Ultimately, the comprehensive results obtained for the year 2060 are as follows:

- ✓ Domestic need: $Q_{max/d} = 53459.85 \text{ m}^3/\text{day}$;
- ✓ Public sector: $Q_{max/d} = 3938.4 \text{ m}^3/\text{day}$;
- ✓ Industrial sector: $Q_{max/d} = 757.37 \text{ m}^3/\text{day}$;
- ✓ Tertiary sector: $Q_{max/d} = 1514.78 \text{ m}^3/\text{day}$;
- ✓ The total sum amounts to $59670.40 \text{ m}^3/\text{day}$, so we can consider $60.000 \text{ m}^3/\text{day}$ as a value for the design of the drinking water supply network.

In the next chapter, hydraulic calculations and sizing of the pipes will be performed.

Chapter IV

Study of the water supply network

IV.1 Introduction

Water supply is the process of getting and transporting potable water from its source to consumers taps. This study focuses on capturing water from the new treatment plant to the distribution reservoirs. In this chapter, we undertake a techno-economic study to select the optimal materials and diameters for various sections of the considered water supply project.

IV.2 Type of drinking water supply

Generally, three types of drinking water supply may be implemented: pumped, gravity, and mixed, In this project, just two types are used, which are:

IV.2.1 Gravity-fed supply

In a gravity-fed water supply system, the intake point is located at an elevation higher than the piezometric level of the receiving structure. Gravity-fed supply can be achieved either through an aqueduct utilizing the natural slope or through a conduit where the flow occurs by gravity[04].

IV.2.2 Pumped supply

In pumped supply, the intake is situated at a lower level than that of the storage reservoir, requiring the fluid to be supplied with energy, which is provided by pumps [04].

IV.3 Choice of materials

The choice of materials for potable water supply networks is crucial to ensure water quality and system durability. Here are some of the most commonly used materials in hydraulics, along with a few factors to consider when selecting these materials:

Table IV.1: Advantages and disadvantages for each material in hydraulic [05]

Materials	Advantages	Disadvantages
HDPE (High-Density Polyethylene)	<ul style="list-style-type: none"> -Corrosion resistant; -Flexible, lightweight, durable; Easy to install; -Good impact resistance; -Good hydraulic properties; -Less sensitive to temperature variations than PVC. 	<ul style="list-style-type: none"> -May be affected by UV rays if not properly protected; -Initial cost slightly higher than PVC; -Large diameters are made to order (not always available).
Ductile iron	<ul style="list-style-type: none"> -Highly resistant to corrosion; -Durable; -Long lifespan; -Ability to withstand high pressures and loads; -Good hydraulic properties; -Low pressure loss. 	<ul style="list-style-type: none"> -Heavy, requires specialized skills for installation and repair; -Initial cost higher than some other materials.
Steel	<ul style="list-style-type: none"> - Long lifespan; -Ability to withstand high pressures and loads; -Suitable for aggressive environments, recyclable. 	<ul style="list-style-type: none"> -High initial cost, requires specialized skills for installation and welding; -Heavier than other materials.

<p>PRV (Polystère Reinforced with Glass Fiber)</p>	<ul style="list-style-type: none"> -Corrosion resistance; -Lightweight; -Mechanical strength; -Long lifespan; -Electrical insulation 	<ul style="list-style-type: none"> -PRV may have a higher initial cost; -More fragile; -Repair difficulty.
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For material selection, we opt for HDPE (High-Density Polyethylene) in cases where diameters are small and pressure is low, while steel material is chosen for larger diameters and for high pressure. HDPE (High-Density Polyethylene) is chosen due to its long lifespan and availability in the Algerian market. As for steel, it is preferred for its strength, especially in diameters above 500 mm, with cathodic protection to shield the pipes from corrosion. Also, steel is chosen when HDPE is not available off-the-shelf and requires a special order.

IV.3.1 The range of diameters for Steel

The following figure illustrates the available diameters for steel material.

Nominal Pipe Size		Outside Diameter (mm)	Nominal Wall Thickness Schedule																
NPS	DN		SCH 5s	SCH 10s	SCH 10	SCH 20	SCH 30	SCH 40s	SCH STD	SCH 40	SCH 60	SCH 80s	SCH XS	SCH 80	SCH 100	SCH 120	SCH 140	SCH 160	SCH XXS
1/8	6	10.3		1.24				1.73	1.73	1.73		2.41	2.41	2.41					
1/4	8	13.7		1.65				2.24	2.24	2.24		3.02	3.02	3.02					
3/8	10	17.1		1.65				2.31	2.31	2.31		3.20	3.20	3.20					
1/2	15	21.3	1.65	2.11				2.77	2.77	2.77		3.73	3.73	3.73				4.78	7.47
3/4	20	26.7	1.65	2.11				2.87	2.87	2.87		3.91	3.91	3.91				5.56	7.82
1	25	33.4	1.65	2.77				3.38	3.38	3.38		4.55	4.55	4.55				6.35	9.09
1 1/4	32	42.2	1.65	2.77				3.56	3.56	3.56		4.85	4.85	4.85				6.35	9.70
1 1/2	40	48.3	1.65	2.77				3.68	3.68	3.68		5.08	5.08	5.08				7.14	11.07
2	50	60.3	1.65	2.77				3.91	3.91	3.91		5.54	5.54	5.54				8.74	11.07
2 1/2	65	73	2.11	3.05				5.16	5.16	5.16		7.01	7.01	7.01				9.53	14.02
3	80	88.9	2.11	3.05				5.49	5.49	5.49		7.62	7.62	7.62				11.13	15.24
3 1/2	90	101.6	2.11	3.05				5.74	5.74	5.74		8.08	8.08	8.08					
4	100	114.3	2.11	3.05				6.02	6.02	6.02		8.56	8.56	8.56		11.13		13.49	17.12
5	125	141.3	2.77	3.40				6.55	6.55	6.55		9.53	9.53	9.53		12.70		15.88	19.05
6	150	168.3	2.77	3.40				7.11	7.11	7.11		10.97	10.97	10.97		14.27		18.26	21.95
8	200	219.1	2.77	3.76		6.35	7.04	8.18	8.18	8.18	10.31	12.70	12.70	12.70	15.09	18.26	20.62	23.01	22.23
10	250	273.1	3.40	4.19		6.35	7.80	9.27	9.27	9.27	12.70	12.70	12.70	15.09	18.26	21.44	25.40	28.58	25.40
12	300	323.9	3.96	4.57		6.35	8.38	9.53	9.53	10.31	14.27	12.70	12.70	17.48	21.44	25.40	28.58	33.32	25.40
14	350	355.6	3.96	4.78	6.35	7.92	9.53	9.53	11.13	15.09		12.70	19.05	23.83	27.79	31.75	35.71		
16	400	406.4	4.19	4.78	6.35	7.92	9.53	9.53	12.70	16.66		12.70	21.44	26.19	30.96	36.53	40.49		
18	450	457.2	4.19	4.78	6.35	7.92	11.13	9.53	14.27	19.05		12.70	23.83	29.36	34.93	39.67	45.24		
20	500	508	4.78	5.54	6.35	9.53	12.70	9.53	15.09	20.62		12.70	26.19	32.54	38.10	44.45	50.01		
22	559	559	4.78	5.54	6.35	9.53	12.70	9.53		22.23		12.70	28.58	34.93	41.28	47.63	53.98		
24	600	610	5.54	6.35	6.35	9.53	14.27	9.53	17.48	24.61		12.70	30.96	38.89	46.02	52.37	59.54		
26	660	660			7.92	12.70		9.53				12.70							
28	700	711			7.92	12.70	15.88	9.53				12.70							
30	762	762	6.35	7.92	7.92	12.70	15.88	9.53				12.70							
32	800	813			7.92	12.70	15.88	9.53	17.48			12.70							
34	884	884			7.92	12.70	15.88	9.53	17.48			12.70							
36	900	914			7.92	12.70	15.88	9.53	19.05			12.70							
38	965	965						9.53				12.70							
40	1000	1016						9.53	12.70										
42	1067	1067				12.70	15.88	9.53	19.05			12.70							
44	1100	1118						9.53				12.70							
46	1168	1168						9.53				12.70							
48	1200	1219						9.53				12.70							

Figure IV.1: Range of diameters for Steel [06]

IV.3.2 The range of diameters for HDPE (High-Density Polyethylene)

Table IV.2: Range of diameters for HDPE [07]

Diameter	NP10		NP16		NP20		NP25	
	EP (mm)	Price TTC (DA)	EP (mm)	Price TTC (DA)	EP (mm)	Price TTC (DA)	EP (mm)	Price TTC (DA)
20	1.9	32	2.0	41	2.3	47	3	48
25	2.0	41	2.3	53.5	3	64	3.5	68
32	2.0	59	3.0	82	3.6	93	4.4	111
40	2.4	89.5	3.7	125	4.5	144	5.5	166
50	3	128	4.6	185	5.6	220	6.9	270
63	3.8	210	5.8	299	7.1	355	8.6	420
75	4.5	297	6.8	420	8.4	500	10.3	590
90	5.4	422	8.2	580	10.1	720	12.3	850
110	6.6	615	10	875	12.3	1096	15.1	1280
125	7.4	790	11.4	1120	14.0	1350	17.1	1620
160	9.5	1260	14.6	1820	17.9	2250	21.9	2650
200	11.9	1900	18.2	2900	22.4	3370	27.4	4100
250	14.8	3050	22.7	4350	27.9	5400	34.2	6400
315	18.7	4850	28.6	7200	35.2	8750	43.1	10300
400	23.7	7500	36.3	11450	44.7	14000	54.7	17250

500	29.7	11900	45.4	17500	55.8	21500		26400
630	37.4	/	57.2	/	/	/	/	/
710	42.1	/	64.5	/	/	/	/	/
800	47.4	/	72.6	/	/	/	/	/

IV.4 The project layout

Google Earth Pro software was used to create the project layout, making sure to follow certain conditions. For example, it was crucial to keep the route as short as possible for economic reasons. The image below shows the layout for lot number 2, For more details and clarity, please refer to annex number 03.



Figure IV.2: The project layout from the Tichi-Haf dam to the distribution network tanks

IV.5 The water supply network diagram

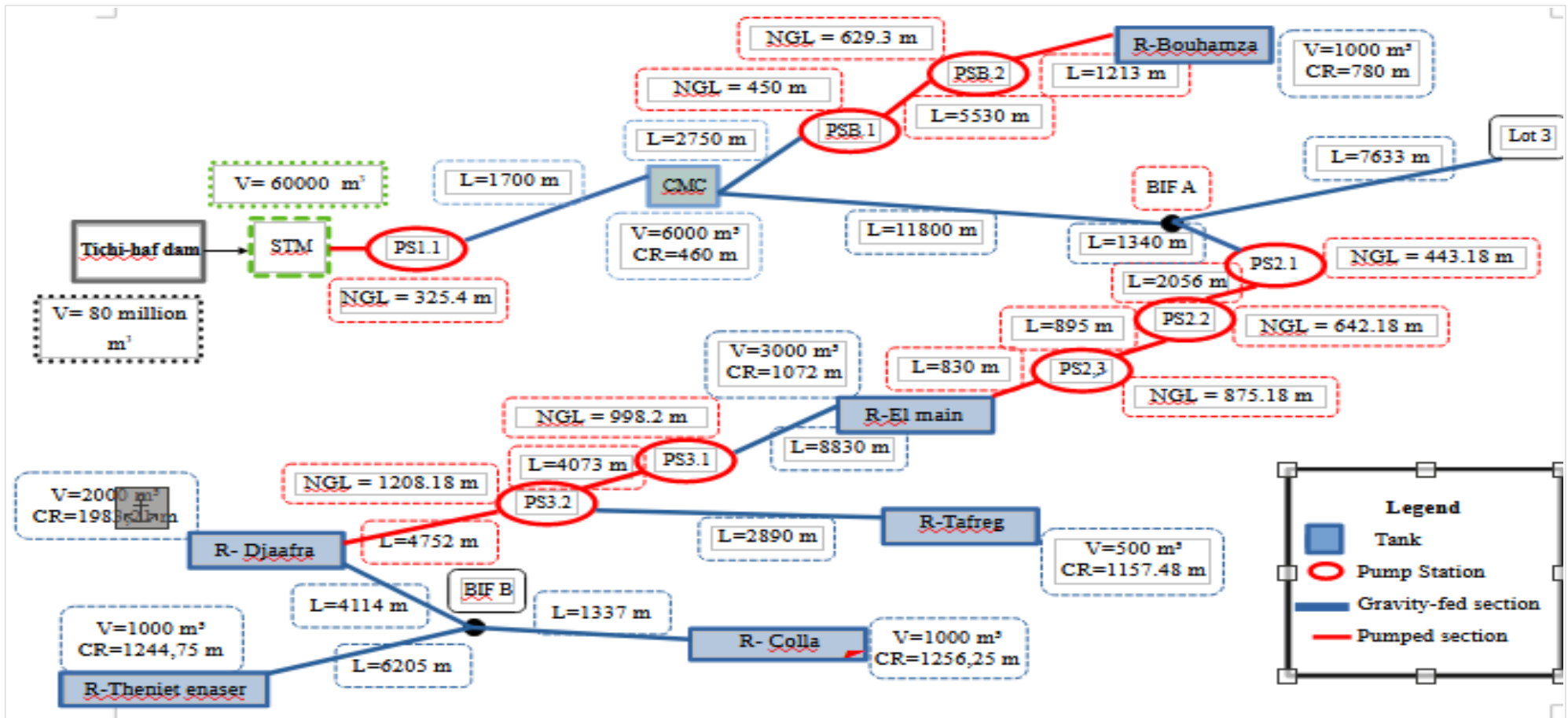


Figure IV.3: The water supply network diagram

IV.6 The values of the flow rates in the pipes

So, from the previous figures, we understand that the project includes 8 pumped pipes and 10 gravity-fed pipes. The flow will be calculated as follows:

$$\rightarrow \text{pumped pipes: } Q(l/s) = \frac{Q(m^3/day) * 10^3}{Tp * 3600} \quad (IV.1)$$

$$\rightarrow \text{gravity-fed pipes: } Q(l/s) = \frac{Q(m^3/day) * 10^3}{24 * 3600} \quad (IV.2)$$

Where:

$Q(l/s)$: Maximum daily flow rate in l/s;

$Q(m^3/day)$: Maximum daily flow rate in (m³/day);

Tp : Time of pumping in h.

Table IV.4: The flow rates in the pipes for lot number 02

Stretch		Type	Flow m ³ /day	Flow l/s
STM	PS1.1	Gravity	60000	694.44
PS1.1	CMC	Pumped	60000	757.58
CMC	PSB.1	Gravity	2786.83	32.25
PSB.1	PSB.2	Pumped	2786.83	38.71
PSB.2	R-BOUHAMZA	Pumped	2786.83	38.71
CMC	Bif-A	Gravity	57213.17	662.19
Bif-A	LOT-03	Gravity	42505.02	491.96
Bif-A	PS2.1	Gravity	14708.15	170.23
PS2.1	PS2.2	Pumped	14708.15	204.28
PS2.2	PS2.3	Pumped	14708.15	204.28
PS2.3	R-ELmain	Pumped	14708.15	204.28
R-ELmain	PS3.1	Gravity	11560.22	133.80
PS3.1	PS3.2	Pumped	11560.22	160.56
PS3.2	R-TAFREG	Gravity	1132.12	13.10
PS3.2	R-DJAAFRA	Pumped	10428.1	144.83
R-DJAAFRA	Bif-B	Gravity	6312.98	73.07
Bif-B	R-COLLA	Gravity	3081.39	35.66
Bif-B	R-THENIET.ENASER	Gravity	3231.58	37.40

IV.6.1 The schematic network in the EPANET software

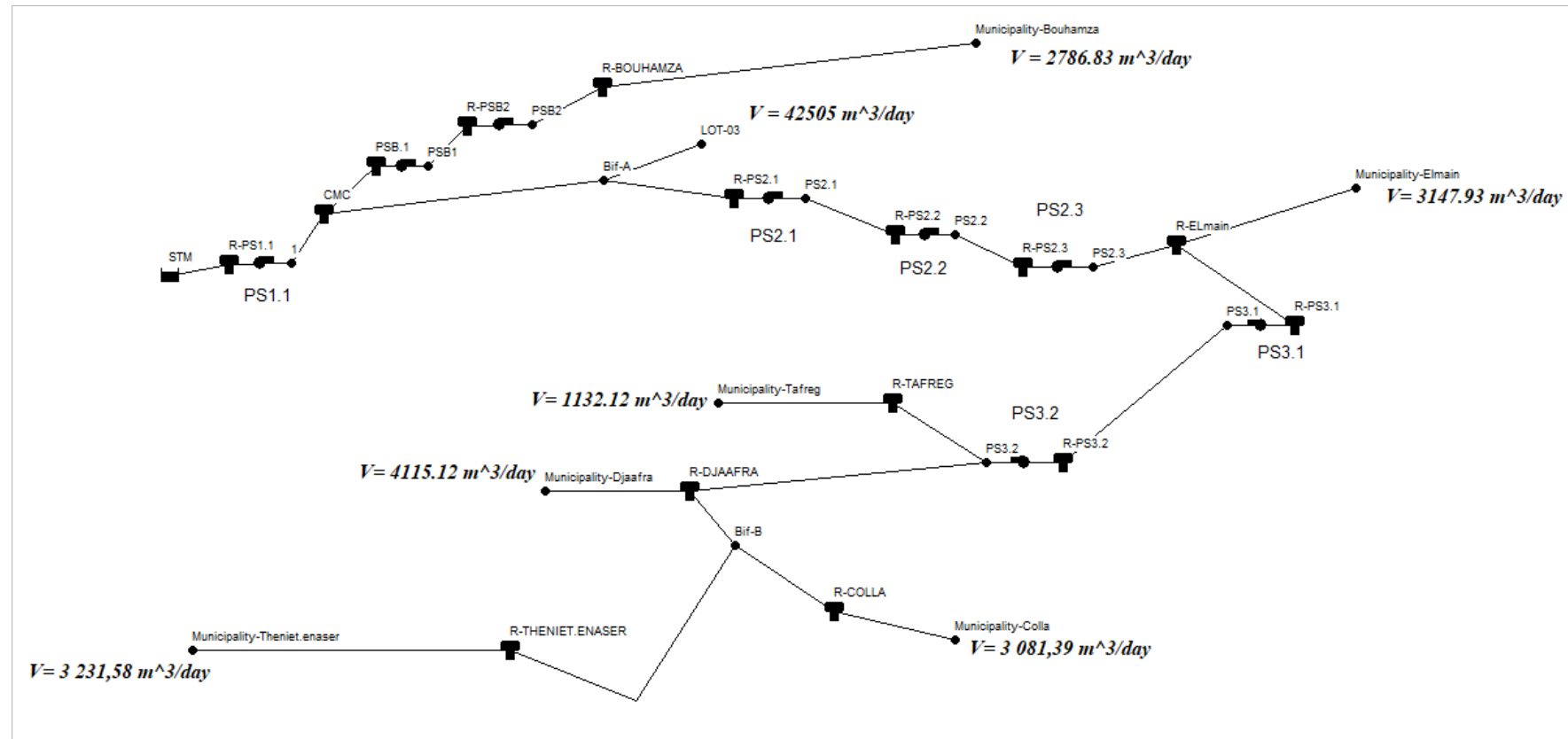


Figure IV.4a: The schematic diagram in the EPANET software

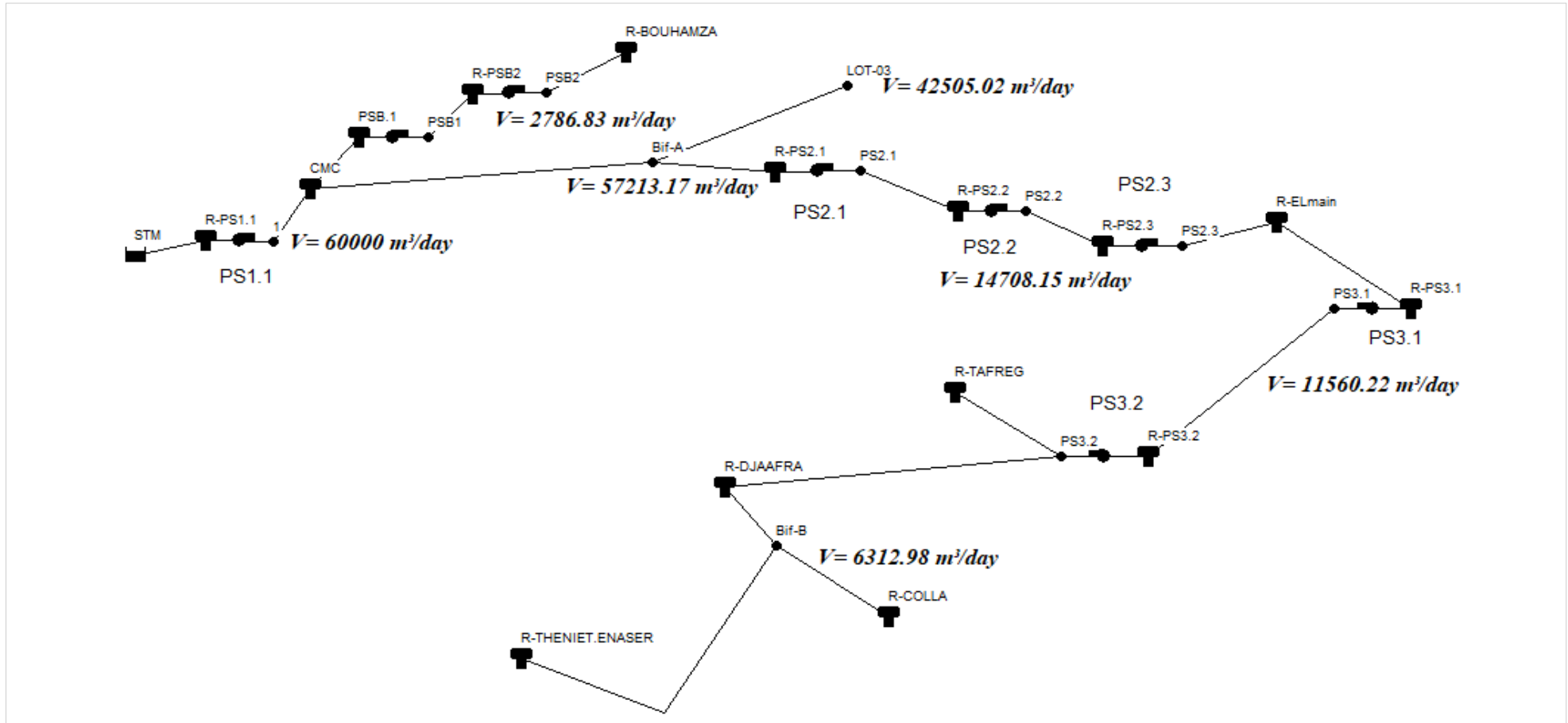


Figure IV.5b: The schematic diagram in the EPANET software

IV.7 Gravity-fed section

IV.7.1 The design laws

IV.7.1.1 Flow rate calculation

The flow (Q) in (m³/s) is calculated using:

$$Q(m^3/s) = Q(l/s) * 10^{-3} \quad (IV.3)$$

IV.7.1.2 Head loss (m)

$$\Delta H = NGL_{UP} - (NGL_{DW} + h) \quad (IV.4)$$

Where:

- ΔH : Head loss in (m);
- NGL_{UP} : Upstream Natural ground level in (m);
- NGL_{DW} : Downstream Natural ground level in (m);
- h : The head of the tank, which is equal to 4m.

IV.7.1.3 Diameter (m)

Diameter is calculated with[04]:

$$D = \frac{\lambda * L * v^2}{2 * \Delta H * g} \quad (IV.5)$$

Where:

- λ : Darcy fraction coefficient;
- L : length of the pipeline (m);
- v : velocity in a pipe in (m/s);
- ΔH : Head loss in (m);
- g : acceleration due to gravity (9.81 m/s²).

Then the nominal diameter D_N is chosen among commercial available ones.

Pipeline cross-section

The cross-section A of a pipe is calculated as follows [04]:

$$A = \frac{\pi * D_{inn}^2}{4} \quad (IV.6)$$

With:

- D_{inn} is pipe inner diameter (m);
- A is cross-section in (m^2).

IV.7.1.4 Velocity

The velocity is calculated as follows [04]:

$$v(\text{m/s}) = \frac{Q(\text{m}^3/\text{s})}{A(\text{m}^2)} \quad (\text{IV.7})$$

The velocity should be between two values: 0.5 m/s to avoid the deposition of sand and other minerals, and 1.5 m/s, It can go up to 2 m/s to avoid pipe vibration.

IV.7.1.5 The linear head losses

The linear head losses are determined using the Darcy-Weisbach formula[04]:

$$\Delta H l = j * L = \frac{\lambda * L * v^2}{2 * D * g} \quad (\text{IV.8})$$

with:

- j : linear head losses in m/ml;
- g : acceleration due to gravity (9.81 m/s^2);
- L : length of the pipeline (m);
- λ : Darcy fraction coefficient.

The calculation of the Darcy coefficient is determined by the following formula:

COLBROOK formula:

$$\frac{1}{\sqrt{\lambda}} = -2 \log \left(\frac{\varepsilon}{3,71 * D} + \frac{2,51}{\Re * \sqrt{\lambda 0}} \right) \quad (\text{IV.9})$$

with:

- ε : pipe roughness coefficient, equal 0.02 mm for HDPE material and 0.1 mm for Steel material;
- Re : Reynolds number.

IV.7.1.6 Singular pressure losses

Singular pressure losses are caused by equipment (valves, air vents, etc...) and are estimated to be 15% of the linear pressure losses.

$$\Delta H_s = \Delta H_l * 0.15 \quad (\text{IV.10})$$

IV.7.1.7 Total head losses

The sum of the two pressure losses ΔH_s and ΔH_l :

$$\Delta H_t = \Delta H_s + \Delta H_l \quad (\text{IV.11})$$

$$\Delta H_t = 1.15 * \Delta H_l \quad (\text{IV.11})$$

IV.7.1.8 The nominal pressure NP

It was calculated using the profile of each pipeline (please refer to the annex number 04 for more details).

Stretch number 01:

Stretch	natural ground level (m)		Hydraulic head (m)		length (m)	Flow m ³ /s	ΔH (m)	Diameter (m)	ND (mm)	Material	Inner diameter (mm)	velocity (m/s)	Lambda λ	Head loss total (m)	NP
	upstream	downstream	upstream	downstream											
STM – PS1.1	332.23	325.4	337.99	331.16	47	0.69	2.83	0.39	500	HDPE	440.6	4.55	0.0136	1.76	NP16
					47	0.69	2.83	0.39	700	Steel	700	1.80	0.0138	0.18	NP16

Discussion : For stretch number 01, extending from the treatment plant station to pumped station 1.1, a nominal diameter of 500 mm with HDPE material has been chosen. The velocity within this pipeline is set to 4.55 m/s. This value was accepted due to the small length of 47 m and to minimize pressure losses. The nominal pressure rating is NP16.

Stretch number 02:

Stretch		natural ground level (m)		Hydraulic head (m)		length (m)	Flow m ³ /s	ΔH (m)	Diameter (m)	ND (mm)	Material	Inner diameter (mm)	velocity (m/s)	Lambda λ	Head loss total (m)	NP
		upstream	downstream	upstream	downstream											
CMC	PSB.1	460	450	469.46	459.46	2750	0.03	6	0.23	250	Steel	250	0.66	0.0196	5.46	NP10

Discussion : For stretch number 02, from the charging chamber to the Bouhamza 1 pumped station, a nominal diameter of 250 mm with Steel material has been chosen. The velocity within this pipeline will be 0.66 m/s, and the nominal pressure rating is NP10.

Stretch number 03:

Stretch		natural ground level (m)		Hydraulic head (m)		length (m)	Flow m ³ /s	ΔH (m)	Diameter (m)	ND (mm)	Material	Inner diameter (mm)	velocity (m/s)	Lambda λ	Total Head loss (m)	NP
		upstream	downstream	upstream	downstream											
CMC – Bif A		460	450	469.46	459.86	11800	0.66	6	1.00	900	Steel	900	1.04	0.0137	9.92	NP25
										1000	Steel	1000	0.84	0.0137	5.86	NP25

Discussion : For stretch number 03, from the charging chamber to the bifurcation point A, a nominal diameter of 1000 mm with Steel material has been chosen. The velocity within this pipeline will be 0.84 m/s, and the nominal pressure rating is NP25.

Stretch number 04:

Stretch		natural ground level (m)		Hydraulic head (m)		length (m)	Flow m ³ /s	ΔH (m)	Diameter (m)	ND (mm)	Material	Inner diameter (mm)	velocity (m/s)	Lambda λ	Head loss total (m)	NP
		upstream	downstream	upstream	downstream											
Bif-A – LOT 03		450	434.35	459.86	445.84	7633	0.49	11.65	0.71	700	Steel	700	1.28	0.0141	14.73	NP16
						7633	0.49	11.65	0.71	800	Steel	800	0.97	0.0141	7.49	NP16
						7633	0.49	11.65	0.71	900	Steel	900	0.77	0.0141	4.16	NP16

Discussion : For stretch number 04, from bifurcation point A to the Lot number 03, a nominal diameter of 800 mm with Steel material has been selected. The velocity within this pipeline will be 0.97 m/s, and the nominal pressure rating is NP16.

Stretch number 05:

Stretch		natural ground level (m)		Hydraulic head (m)		length (m)	Flow m ³ /s	ΔH (m)	Diameter (m)	ND (mm)	Material	Inner diameter (mm)	velocity (m/s)	Lambda λ	Head loss total (m)	NP
		upstream	downstream	upstream	downstream											
Bif-A	PS2.1	450	443.18	459.86	450.32	1340	0.17	3.82	0.42	450	Steel	450	1.07	0.0157	3.14	NP16

Discussion : For stretch number 05, from point A to the pumped station PS2.1, a nominal diameter of 450 mm with Steel material has been selected. The velocity within this pipeline will be 1.07 m/s, and the nominal pressure rating is NP16.

Stretch number 06:

Stretch		natural ground level (m)		Hydraulic head (m)		length (m)	Flow m ³ /s	ΔH (m)	Diameter (m)	ND (mm)	Material	Inner diameter (mm)	velocity (m/s)	Lambda λ	Head loss total (m)	NP
		upstream	downstream	upstream	downstream											
R-ELmain	PS3.1	1072	998.2	1,099.62	1,025.82	8830	0.13	69.8	0.31	300	Steel	300	1.89	0.0164	101.37	
						8830	0.13	69.8	0.31	400	Steel	400	1.06	0.0161	23.62	NP25

Discussion : For stretch number 06, from the El-main reservoir to the pumped station PS3.1, a nominal diameter of 400 mm with Steel material has been selected. The velocity within this pipeline will be 1.06 m/s, and the nominal pressure rating is NP25.

Stretch number 07:

Stretch		natural ground level (m)		Hydraulic head (m)		length (m)	Flow m ³ /s	ΔH (m)	Diameter (m)	ND (mm)	Material	Inner diameter (mm)	velocity (m/s)	Lambda λ	Head loss total (m)	NP
		upstream	downstream	upstream	downstream											
SP3.2	R-TAFREG	1208.18	1157.48	1,234.35	1,183.65	2890	0.01	46.7	0.11	125	HDPE	102.2	1.60	0.0176	74.43	
						2890	0.01	46.7	0.11	160	HDPE	130.8	0.98	0.018	22.17	NP16

Discussion : For stretch number 07, from the pumped station PS3.2 to the Tafreg reservoir, a nominal diameter of 160 mm with HDPE material has been selected. The velocity within this pipeline will be 0.98 m/s, and the nominal pressure rating is NP16.

Stretch number 08:

Stretch		natural ground level (m)		Hydraulic head (m)		length (m)	Flow m ³ /s	ΔH (m)	Diameter (m)	ND (mm)	Material	Inner diameter (mm)	velocity (m/s)	Lambda λ	Linear Head loss (m)	NP
		upstream	downstream	upstream	downstream											
R-DJAAFRA	Bif-B	1383.21	1300	1,482.79	1,399.58	4114	0.07	83.21	0.20	200	Steel	200	2.33	0.0148	83.11	NP20

Discussion : For stretch number 08, from the Djaafra reservoir to the bifurcation point B, a nominal diameter of 200 mm with steel material. The velocity within this pipeline will be 2.33 m/s, and the nominal pressure rating is NP20.

Stretch number 09:

Stretch		natural ground level (m)		Hydraulic head (m)		length (m)	Flow m ³ /s	ΔH (m)	Diameter (m)	ND (mm)	Material	Inner diameter (mm)	velocity (m/s)	Lambda λ	Total Head loss (m)	NP
		upstream	downstream	upstream	downstream											
Bif-B	R-COLLA	1300	1256.25	1,399.58	1,300.89	1337	0.04	39.75	0.14	150	Steel	150	2.02	0.0191	35.34	NP10

Discussion : For stretch number 09, from the bifurcation point B to the Colla Tank, a nominal diameter of 150 mm with steel material has been selected. The velocity within this pipeline will be 2.02 m/s, and the nominal pressure rating is NP10.

Stretch number 10:

Stretch		natural ground level (m)		Hydraulic head (m)		length (m)	Flow m ³ /s	ΔH (m)	Diameter (m)	ND (mm)	Material	Inner diameter (mm)	velocity (m/s)	Lambda λ	Head loss total	NP
		upstream	downstream	upstream	downstream											
Bif-B	HENIET.ENAS	1300	1244.75	1,399.58	1,296.44	6205	0.04	51.25	0.19	200	Steel	200	1.19	0.0185	47.69	NP16

Discussion : For stretch number 10, from the bifurcation B to the Theniet enaser reservoir, a nominal diameter of 200 mm with steel material. The velocity within this pipeline will be 1.19 m/s, and the nominal pressure rating is NP16.

IV.8 Pumped section

IV.8.1 The design laws

IV.8.1.1 Economic diameter

The economic diameter was calculated using the following formula [08]:

✓ BOUNIN formula (m): $D(BOUNIN) = \sqrt{Q}$ (IV.13)

✓ BRESS formula (m): $D(BRESS) = 1,5 * \sqrt{Q}$ (IV.14)

with:

- D_{BOUNIN}, D_{BRESS} : Pipeline diameter (m);
- Q : Flow rate passing through the section (m³/s).

These two formulas provide two values for the diameter. Then we choose the best one based on velocity and other hydraulic parameters.

IV.8.1.2 Geometric head

Represented by the following formula[04]:

$$Hg = NGL_{UP} - NGL_{DW} \quad (IV.15)$$

IV.8.1.3 Total dynamic head

Represented by the following formula[04]:

$$TDH(m) = Hg + \Delta H_t \quad (IV.16)$$

with:

- Hg : geometric head (m);
- ΔH_t : total head losses (m).

IV.8.1.4 The nominal pressure NP

It was calculated using the profile of each conduit, for more details please refer to the annex number 04.

Stretch number 01: PS1.1 - CMC

Stretch	natural ground level (m)		length (m)	Flow m ³ /s	Diameter BOUNIN (m)	ND (mm)	Diameter BRESS (m)	ND (mm)	ND (mm)	Material	Inner diameter (mm)	Cross section (m ²)	velocity (m/s)	Hg (m)	Lambda λ	Linear Head loss (m)	Head loss total	TDH (m)	NP
	upstream	downstream																	
PS1.1 – CMC	325.4	460	1700	0.76	0.87	900	1.31	1300	800	Steel	800	0.50	1.51	134.6	0.0136	3.35	3.85	138.45	NP40
									900	Steel	900	0.64	1.19	135.6	0.0135	1.84	2.12	137.72	
									1000	Steel	1000	0.79	0.96	134.6	0.0135	1.09	1.25	135.85	
									1100	Steel	1100	0.95	0.80	134.6	0.0135	0.68	0.78	135.38	
									1200	Steel	1200	1.13	0.67	134.6	0.0135	0.44	0.50	135.10	
									1300	Steel	1300	1.33	0.57	134.6	0.0135	0.29	0.34	134.94	

For stretch number 01, from the pumped station PS1.1 to the charging chamber, the nominal diameter of 800 mm is chosen with Steel material. The velocity within this pipeline will be 1.51 m/s, and the nominal pressure rating is NP40.

Stretch number 02: PSB.1 – PSB.2

Stretch	natural ground level (m)		length (m)	Flow m ³ /s	Diameter BOUNIN (m)	ND (mm)	Diameter BRESS (m)	ND (mm)	ND (mm)	Material	Inner diameter (mm)	Cross section (m ²)	velocity (m/s)	Hg (m)	Lambda λ	Linear Head loss (m)	Head loss total	TDH (m)	NP
	upstream	downstream																	
PSB.1 – PSB.2	458.25	629.3	5530	0.04	0.20	200	0.30	300	200	Steel	200	0.03	1.23	171.05	0.0155	33.16	38.13	209.18	NP40
									250	Steel	250	0.05	0.79	171.05	0.0157	11.01	12.66	183.71	
									300	Steel	300	0.07	0.55	171.05	0.0161	4.54	5.22	176.27	

For stretch number 02, from Bouhamza Pumped Station 1 to Bouhamza Pumped Station 2, the nominal diameter of 200 mm is chosen with Steel material. The velocity within this pipeline will be 1.23 m/s, resulting in a total dynamic head of 209.18 m. The nominal pressure rating is NP40.

Stretch number 03: PSB.2 – R-Bouhamza

Stretch	natural ground level (m)		length (m)	Flow m ³ /s	Diameter BOUNIN (m)	ND (mm)	Diameter BRESS (m)	ND (mm)	ND (mm)	Matrerial	Inner diameter (mm)	Cross section (m ²)	velocity (m/s)	Hg (m)	Lambda λ	Linear Head loss (m)	Head loss total	TDH (m)	NP
	upstream	downstream																	
PSB.2 – R-BOUHAMZA	629.3	780	1213	0.04	0.20	200	0.30	300	200	Steel	200	0.03	1.23	150.7	0.0155	7.27	8.36	159.06	NP40
									250	Steel	250	0.05	0.79	150.7	0.0157	2.41	2.78	153.48	
									300	Steel	300	0.07	0.55	150.7	0.0161	0.99	1.14	151.84	

For stretch number 03, from Bouhamza Pumped Station 2 to Bouhamza reservoir, the nominal diameter of 200 mm is chosen with Steel material. The velocity within this pipeline will be 1.23 m/s, resulting in a total dynamic head of 159.06 m. The nominal pressure rating is NP40.

Stretch number 04: PS2.1 – PS2.2

Stretch	natural ground level (m)		length (m)	Flow m ³ /s	Diameter BOUNIN (m)	ND (mm)	Diameter BRESS (m)	ND (mm)	ND (mm)	Matrerial	Inner diameter (mm)	Cross section (m ²)	velocity (m/s)	Hg (m)	Lambda λ	Linear Head loss (m)	Head loss total	TDH (m)	NP
	upstream	downstream																	
PS2.1 –PS2.2	443.18	642.18	2056	0.20	0.45	450	0.68	700	450	Steel	450	0.16	1.28	199	0.0155	5.95	6.85	205.85	NP40
									500	Steel	500	0.20	1.04	199	0.0154	3.49	4.02	203.02	
									600	Steel	600	0.28	0.72	199	0.0153	1.39	1.60	200.60	
									700	Steel	700	0.38	0.53	199	0.0154	0.65	0.75	199.75	

For stretch number 04, from pumped station 2.1 to the pumped station 2.2, the nominal diameter of 450 mm is chosen with Steel material. The velocity within this pipeline will be 1.28 m/s, resulting in a total dynamic head of 205.85 m. The nominal pressure rating is NP40.

Stretch number 05: PS2.2 – PS2.3

Stretch	natural ground level (m)		length (m)	Flow m ³ /s	Diameter BOUNIN (m)	ND (mm)	Diameter BRESS (m)	ND (mm)	ND (mm)	Material	Inner diameter (mm)	Cross section (m ²)	velocity (m/s)	Hg (m)	Lambda λ	Linear Head loss (m)	Head loss total	TDH (m)	NP
	upstream	downstream																	
PS2.2 – PS2.3	642.18	875.18	895	0.20	0.45	450	0.68	700	450	Steel	450	0.16	1.28	233	0.0155	2.59	2.98	235.98	NP40
									500	Steel	500	0.20	1.04	233	0.0154	1.52	1.75	234.75	
									600	Steel	600	0.28	0.72	233	0.0153	0.61	0.70	233.70	
									700	Steel	700	0.38	0.53	233	0.0154	0.28	0.33	233.33	

For stretch number 05, from pumped station 2.2 to the pumped station 2.3, the nominal diameter of 450 mm is chosen with Steel material. The velocity within this pipeline will be 1.28 m/s, resulting in a total dynamic head of 235.98 m. The nominal pressure rating is NP40.

Stretch number 06: PS2.3 – R-E Imain

Stretch	natural ground level (m)		length (m)	Flow m ³ /s	Diameter BOUNIN (m)	ND (mm)	Diameter BRESS (m)	ND (mm)	ND (mm)	Material	Inner diameter (mm)	Cross section (m ²)	velocity (m/s)	Hg (m)	Lambda λ	Linear Head loss (m)	Head loss total	TDH (m)	NP
	upstream	downstream																	
PS2.3 – R-Elmain	875.18	1072	830	0.20	0.45	450	0.68	700	450	Steel	450	0.16	1.28	196.82	0.0155	2.40	2.76	199.58	NP40
									500	Steel	500	0.20	1.04	196.82	0.0154	1.41	1.62	198.44	
									600	Steel	600	0.28	0.72	196.82	0.0153	0.56	0.65	197.47	
									700	Steel	700	0.38	0.53	196.82	0.0154	0.26	0.30	197.12	

For stretch number 06, from pumped station 2.3 to Elmain reservoir, the nominal diameter of 450 mm is chosen with Steel material. The velocity within this pipeline will be 1.28 m/s, resulting in a total dynamic head of 199.58 m. The nominal pressure rating is NP40.

Stretch number 07: PS3.1 – PS3.2

Stretch	natural ground level (m)		length (m)	Flow m ³ /s	Diameter BOUNIN (m)	ND (mm)	Diameter BRESS (m)	ND (mm)	ND (mm)	Material	Inner diameter (mm)	Cross section (m ²)	velocity (m/s)	Hg (m)	Lambda λ	Linear Head loss (m)	Head loss total	TDH (m)	NP
	upstream	downstream																	
PS3.1 – PS3.2	998.2	1208.18	4073	0.16	0.40	400	0.60	600	350	Steel	350	0.10	1.67	209.98	0.0138	22.80	26.21	236.19	
									400	Steel	400	0.13	1.28	209.98	0.0158	13.39	15.39	225.37	NP40
									450	Steel	450	0.16	1.01	209.98	0.0157	7.38	8.49	218.47	
									500	Steel	500	0.20	0.82	209.98	0.0157	4.36	5.01	214.99	
									600	Steel	600	0.28	0.57	209.98	0.0158	1.76	2.03	212.01	

For stretch number 07, from the pumped station 3.1 to pumped station 3.2, the nominal diameter of 400 mm is chosen with Steel material. The velocity within this pipeline will be 1.28 m/s, resulting in a total dynamic head of 225.37 m. The nominal pressure rating is NP40.

Stretch number 08: PS3.2 - R-Djaafra

Stretch	natural ground level (m)		length (m)	Flow m ³ /s	Diameter BOUNIN (m)	ND (mm)	Diameter BRESS (m)	ND (mm)	ND (mm)	Material	Inner diameter (mm)	Cross section (m ²)	velocity (m/s)	Hg (m)	Lambda λ	Linear Head loss (m)	Head loss total	TDH (m)	PN
	upstream	downstream																	
PS3.2 – R-DJAAFRA	1208.18	1383.21	4752	0.14	0.38	400	0.57	600	350	HDPE	350	0.10	1.51	175.03	0.0164	25.72	29.58	204.61	
									400	Steel	400	0.13	1.15	175.03	0.016	12.87	14.80	189.83	NP40
									450	Steel	450	0.16	0.91	175.03	0.0159	7.10	8.16	183.19	
									500	Steel	500	0.20	0.74	175.03	0.0159	4.19	4.82	179.85	
									600	Steel	600	0.28	0.51	175.03	0.0159	1.68	1.94	176.97	

For stretch number 08, from the pumped station 3.2 to Djaafra reservoir, the nominal diameter of 400 mm is chosen with Steel material. The velocity within this pipeline will be 1.15 m/s, resulting in a total dynamic head of 189.83 m. The nominal pressure rating is NP40.

Tableau IV.5: Summary of the results for LOT number 02

IV.9 Summary of the results

Stretch		Length (m)	Type of supply	Flow m ³ /s	ND (mm)	Material	Velocity (m/s)	NP
STM	PS1.1	4312	gravity	0.694	500	HDPE	4.55	NP16
PS1.1	CMC	1700	Pumped	0.757	800	Steel	1.15	NP40
CMC	PSB.1	2750	gravity	0.032	250	Steel	0.66	NP10
PSB.1	PSB.2	5530	Pumped	0.039	200	Steel	1.23	NP40
PSB.2	R-BOUHAMZA	1213	Pumped	0.039	200	Steel	1.23	NP40
CMC	Bif-A	11800	gravity	0.663	100	Steel	0.84	NP25
Bif-A	LOT-03	7633	gravity	0.492	800	Steel	0.97	NP16
Bif-A	PS2.1	1340	gravity	0.17	450	Steel	1.07	NP16
PS2.1	PS2.2	2056	Pumped	0.204	450	Steel	1.28	NP40
PS2.2	PS2.3	895	Pumped	0.204	450	Steel	1.28	NP40
PS2.3	R-ELmain	830	Pumped	0.204	450	Steel	1.28	NP40
R-ELmain	PS3.1	8830	gravity	0.134	400	Steel	1.06	NP25
PS3.1	PS3.2	4073	Pumped	0.161	400	Steel	1.28	NP40
PS3.2	R-TAFREG	2890	gravity	0.013	160	HDPE	0.98	NP16
PS3.2	R-DJAAFRA	4752	Pumped	0.145	400	Steel	1.48	NP40
R-DJAAFRA	Bif-B	4114	gravity	0.073	200	Steel	1.15	PN20
Bif-B	R-COLLA	1337	gravity	0.036	150	Steel	2.02	PN10
Bif-B	R-THENIET.ENASER	6205	gravity	0.037	200	Steel	1.19	NP16

IV.10 Conclusion

To summarize, this chapter covers the following points:

- ✓ The choice of materials, where steel was preferred over HDPE in certain cases, despite the smaller diameter, to optimize other hydraulic parameters;
- ✓ Presentation of the project layout using Google Earth Pro and Epanet software;
- ✓ Detailed study of the water supply network, including calculations of hydraulic parameters such as diameter, velocity, nominal pressure, total head loss, and total dynamic head for both gravity-fed and pumped sections;
- ✓ Following to the pipeline study, a pump selection study for the pumping section is required.

Chapter V
Pump Selection and
Water Hammer Effect

V.1 Introduction

Pumps are essential mechanical devices in hydraulics. Their crucial role is to transfer water from one place to another. In this chapter, we will examine the different types of pumps and how we made the selection for the various pumping stations.

V.2 What is a pump?

A pump is a mechanical device that is used to give energy to a flowing liquid such that the liquid can overcome the resistance in the hydraulic system. Simply the pump converts mechanical energy into hydraulic energy given to the flow [09].

V.3 Types of pumps

V.3.1 Reciprocation Pumps

The pressure energy of fluid increase due to positive displacement of its piston or plunger. These are used to the handle low increase the rate at high pressure [09].

V.3.2 Rotary Displacement pumps

Combine the advantages of reciprocating and centrifugal pump. They are positive in action, compact, produce an even flow, have no valves and run at high speeds. These pumps are suitable for handling oils but they are unsuitable for gritty (sandy) liquid [09].

V.3.3 Centrifugal pump

It is by far the commonest type of dynamic pump also called as velocity pump. These are classified as roto-dynamic pumps since the rotating impeller of pump impresses a centrifugal head or pressure on the liquid which leaves the impeller at a high velocity. This pressure allows the liquid to rise to a higher level [09].

V.4 Pump coupling

In this study, it was found that one pump is not sufficient; thus, coupled pumps are required, either in parallel or in series, according to the needs identified.

V.4.1 Series coupling

Significantly increasing the Pumped head, which is the total dynamic head.

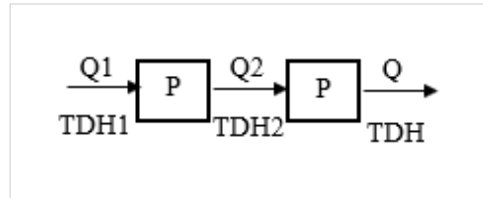


Figure V.1: Mounting two (02) pumps in series

V.4.2 Parallel coupling

Significantly increasing the pumped flow rate in a network.

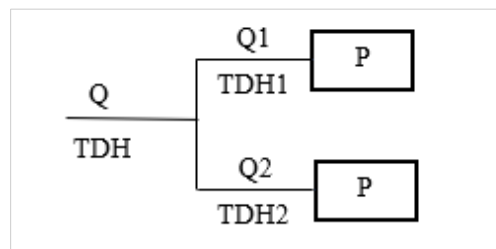


Figure V.2: Mounting two (02) pumps in parallel

V.5 The pump characteristic

- The operating point (flow and total dynamic head);
- Optimal efficiency;
- Non-cavitation condition verified $NPSHr < NPSHd$;
- Low electricity or energy consumption.

V.6 The absorbed power of the pumps

Calculated following this formula [10]:

$$P_{ab} = \frac{P_{sh}}{\eta} = \frac{Q * TDH * g}{\eta} \quad (V.1)$$

where:

- TDH : Total Dynamic Head (m);
- P_{ab} : absorbed power of the pumps (KW);
- P_{sh} : shaft power (KW);

- Q : pump flow rate (m³/s);
- g : acceleration due to gravity (m/s²);
- η : pump efficiency (%).

V.7 Energy consumed by a pump for one year of operation

Calculated following this formula [10]:

$$E = Pab * T * 365 \quad (\text{V.2})$$

with

- E : energy consumed by the pump for one year of operation (kw.h);
- Pab : absorbed power of the pumps (KW);
- T : time pumping (h).

V.8 Checking pump cavitation

The NPSH (Net Positive Suction Head) is a criterion used to define the pressure required for pump suction. The condition of no cavitation is verified if [10]:

$$NPSH_d > NPSH_r$$

where:

- $NPSH_d$: Net Positive Suction Head (in, meter water column ‘mwc’);
- $NPSH_r$: Net Positive Suction Head (in mwc).

NPSH in pumping stations is calculated as follows:

$$NPSH_d = Patm - Pv - \Delta H_{asp} + Hasp \quad (\text{V.3}) \quad (\text{Suction under load})$$

Where:

- $Patm$: atmospheric pressure in meters of head (mwc);
- Pv : absolute vaporization pressure of the fluid (0.24 mwc);
- ΔH_{asp} : suction pipe head losses in meters of head (mwc);

$$\Delta H_{asp} = \frac{\lambda * L_{asp} * V^2}{2 * D * g} \quad (\text{V.4})$$

with:

- L_{asp} : the length of aspiration (mwc);
- $Hasp$: Suction head (mwc).

Remark:

We opted for the LOWARA and GOULDS water technologie a Xylem brand (<https://www.xylem.com/fr-fr/support/outils-et-calculateurs-interactifs/xylect/>). The brand offers an easy-to-use web portal for pump selection. In most pumping stations, we choose to couple pumps in series because we have not found pumps that meet the required specifications (the operating point). Of course, we always have another pump in reserve in case of breakdown or for repair work.

V.9 Pump selection**PS1.1 pumping station**

At this station, 2 pumps were chosen for a parallel circuit, with one pump serving as a reserve. The type found is e-XC350-690/8000W/W45ADS4AG, and cavitation is monitored due to $NPSH_d > NPSH_r$.

Tableau V.1: Characteristics of pumping station 1.1

PS 1.1	Naturel ground level	ND (mm)	Time of pumping	Flow m ³ /h	TDH(m)	Hg (m)	ΔH as (m)	NPSHd (m)	Effeciency (%)	NPSHr (m)	shaft power (kw)	Pab (kw)	E (kw,h)
	325.4	800	22	2500	138.45	134.6	0.002	11.77	76.5	4.36	1246.2	1,232.93	9900419.24

PSB.1 pumping station

At this station, 2 pumps were chosen for a serial circuit, with one pump serving as a reserve. The type found is IXPC125-100-315D750L25BDN4S1G, and cavitation is monitored due to $NPSH_d > NPSH_r$.

Tableau V.2: Characteristics of pumping station B.1

PSB.1	Naturel ground level (m)	ND (mm)	Time of pumping	Flow m ³ /h	TDH(m)	Hg (m)	ΔH as (m)	NPSHd (m)	Effeciency (%)	NPSHr (m)	shaft power (kw)	Pab (kw)	E (kw,h)
	450	200	20	116.12	209.18	171.05	0.006	11.77	57.6	3.45	115.2	114.913541	838868.852

PSB.2 pumping station

At this station, 3 pumps were chosen for a parallel circuit, with one pump serving as reserve. The type found is MPA100A/08A/BD750/L45VCCC4, and cavitation is monitored due to $NPSH_d > NPSH_r$.

Tableau V.3: Characteristics of pumping station B.2

PSB.2	Naturel ground level (m)	ND (mm)	Time of pumping	Flow m ³ /h	TDH(m)	Hg (m)	ΔH as (m)	NPSHd (m)	Effeciency (%)	NPSHr (m)	shaft power (kw)	Pab (kw)	E (kw,h)
	629.3	200	20	116.12	159.06	150.7	0.006	11.77	67.8	1.09	76.9	74.23	541.91

SP2.1 pumping station

At this station, 4 pumps were chosen for a parallel circuit, with one pump serving as reserve. The type found is MPA125A/02A/BD1600/L25VCCC4, and cavitation is monitored due to $NPSH_d > NPSH_r$.

Tableau V.4: Characteristics of pumping station 2.1

PS2.1	Naturel ground level (m)	ND (mm)	Time of pumping	Flow m ³ /h	TDH(m)	Hg (m)	ΔH as (m)	NPSHd (m)	Effeciency (%)	NPSHr (m)	shaft power (kw)	Pab (kw)	E (kw,h)
	443.18	450	20	612.84	205.85	199	0.003	11.77	76.4	4.47	449.3	449.96	3284687

SP2.2 pumping station

At this station, 3 pumps were chosen for a parallel circuit, with one pump serving as reserve. The type found is MPA150A/02C/BD3150/L25VDNC4, and cavitation is monitored due to $NPSH_d > NPSH_r$.

Tableau V.5: Characteristics of pumping station 2.2

PS2.2	Naturel ground level (m)	ND (mm)	Time of pumping	Flow m ³ /h	TDH(m)	Hg (m)	ΔH as (m)	NPSHd (m)	Effeciency (%)	NPSHr (m)	shaft power (kw)	Pab (kw)	E (kw,h)
	642.18	450	20	612.84	235.85	233	0.003	11.77	72.3	6.66	536.3	544.77	3976803

SP2.3 pumping station

At this station, 4 pumps were chosen for a parallel circuit, with one pump serving as reserve. The type found is MPA125B/02C/BD1600/L25VCCC4, and cavitation is monitored due to $NPSH_d > NPSH_r$.

Tableau V.6: Characteristics of pumping station 2.3

PS2.3	Naturel ground level (m)	ND (mm)	Time of pumping	Flow m ³ /h	TDH(m)	Hg (m)	ΔH as (m)	NPSH _d (m)	Effeciency (%)	NPSH _r (m)	shaft power (kw)	Pab (kw)	E (kw,h)
	875.18	450	20	612.84	199.58	196.82	0.003	11.77	72.6	4.84	461	459.09	3351327

SP3.1 pumping station

At this station, 4 pumps were chosen for a serial circuit, with one pump serving as reserve. The type found is PA 154/2-SA111-16004, and cavitation is monitored due to $NPSH_d > NPSH_r$.

Tableau V.7: Characteristics of pumping station 3.1

PS3.1	Naturel ground level (m)	ND (mm)	Time of pumping	Flow m ³ /h	TDH(m)	Hg (m)	ΔH as (m)	NPSH _d (m)	Effeciency (%)	NPSH _r (m)	shaft power (kw)	Pab (kw)	E (kw,h)
	998.2	400	20	481.68	225.37	209.98	0.003	11.77	79	6.12	386.4	374.45	2733487

SP3.2 pumping station

At this station, 5 pumps were chosen for a parallel circuit, with one pump serving as reserve. The type found is MPA65B/04A/BD750/L25VCCC4, and cavitation is monitored due to $NPSH_d > NPSH_r$.

Tableau V.8: Characteristics of pumping station 3.2

PS3.2	Naturel ground level (m)	ND (mm)	Time of pumping	Flow m ³ /h	TDH(m)	Hg (m)	ΔH as (m)	NPSH _d (m)	Effeciency (%)	NPSH _r (m)	shaft power (kw)	Pab (kw)	E (kw,h)
	1208.18	400	20	434.5	189.93	175.03	0.003	11.77	79.1	4.1	279.5	284.30	2075373

Remark:

Please refer to the annex number 05, for more details about pump curves for each pumping station.

V.10 Water hammer effect

The water hammer effect refers to the sudden stoppage of flow in a pipe system, typically caused by the abrupt closing of a valve, faucet, or the starting or stopping of a pump, leading to a change in liquid velocity [11].

V.10.1 The wave velocity

The wave velocity, given by the following value [09]:

$$a = \frac{9900}{\sqrt{48.3 + \frac{K * D}{e}}} \quad (V.5)$$

with

- a : Wave velocity in m/s;
- k : Coefficient dependent on the material of the conduit;
- D : Inner diameter of the conduit, in m;
- e : Thickness of the pipe, in m.

Table V.9: The ranges of wave velocity and the values of coefficient K for the two materials[09].

Material	K	Range of wave velocity (m/s)
Steel	0.5	900-1250
HDPE	83	230-430

V.10.2 Value of water hammer**Stop time of the pump**

Given by the following formula [12]:

$$T_A(s) = \frac{L \cdot V_0}{[9,81 \cdot (HMT + P_{atm} - 3)]} \quad (V.6)$$

If $T < \frac{2L}{a}$ case of sudden closer:

The value of water hammer Given by the following value [09]:

$$B = \frac{a \cdot V_0}{g} \quad (V.7)$$

with:

- B : Maximum value of water hammer in m;
- V_0 : Flow velocity (m/s);
- g : acceleration due to gravity (9.81 m/s²).

If $T > \frac{2L}{a}$ Case of slow closer:

The value of water hammer Given by the following value [09]:

$$B = \frac{2LV_0}{gT} \quad (V.8)$$

with:

- B : Maximum value of water hammer in m;
- V_0 : Flow velocity (m/s);
- g : acceleration due to gravity (9.81 m/s²);
- L : length of the pipeline (m);
- T : Stop time of the pump (s).

V.10.3 Surges and depressions in the Pumped pipeline

We use the following formulas for calculation:

$$P_{max} = H_g + B \quad (V.9)$$

$$P_{min} = H_g - B \quad (V.10)$$

with:

- P_{max} : Surge value (m);
- P_{min} : Depression value (m);

- H_g : geometric head (m).

V.10.4 Atmospheric pressure

Given by the following formula [08]:

$$P_{atm}(m) = 10,21 - 0,001 \cdot \text{Altitude}(m) \quad (\text{V.10})$$

V.10.5 Calculation results of surge and depression for each pipeline

Tableau V.10: Surge and depression for each pipeline

Stretch		Hg (m)	ND (m)	Material	L (m)	Inner diameter (mm)	e (mm)	K	a (m/s)	Patm	T (s)	2L/a (s)	Type	V0 (m/s)	B (m)	Pmax (m)	Pmin (m)	NP
PS1.1	CMC	134.6	900	Steel	1700	800	7.1	0.5	967.81	9.88	1.44	3.51	Sudden closure	1.51	148.97	283.57	-14.37	NP40
PSB.1	PSB.2	171.05	250	Steel	5530	200	3.2	0.5	1,109.98	9.76	4.81	9.96	Sudden closure	1.23	139.17	310.22	31.88	
PSB.2	R-BOUHAMZA	150.7	250	Steel	1213	200	3.2	0.5	1,109.98	9.58	1.48	2.19	Sudden closure	1.23	139.17	289.87	11.53	
PS2.1	PS2.2	199	450	Steel	2056	450	4.5	0.5	998.52	9.77	1.03	4.12	Sudden closure	1.28	130.29	329.29	68.71	
PS2.2	PS2.3	233	450	Steel	895	450	4.5	0.5	998.52	9.57	0.39	1.79	Sudden closure	1.28	130.29	363.29	102.71	
PS2.3	R-ELmain	196.82	450	Steel	830	450	4.5	0.5	998.52	9.33	0.17	1.66	Sudden closure	1.28	130.29	327.11	66.53	
PS3.1	PS3.2	209.98	400	Steel	4073	400	4	0.5	998.52	9.21	1.47	8.16	Sudden closure	1.28	130.29	340.27	79.69	
PS3.2	R-DJAAFRA	175.03	400	Steel	4752	400	4	0.5	998.52	9.00	5.39	9.52	Sudden closure	1.15	117.05	292.08	57.98	

Discussion:

In this table, we observe that the wave velocity, for steel, fall within the expected range.

The P_{max} values reveal that all pipelines are under the nominal pressure value, indicating no risk of water hammer phenomenon.

However, the P_{min} values do not exceed the nominal pressure value. Additionally, the depression values obtained are mostly positive, except for the first pipeline which is -1.44bar. Therefore, there is no risk of pipeline destruction due to the water hammer phenomenon.

V.11 Water hammer analysis using the envelope method

This method allows for determining precisely the areas prone to flattening due to depressions and areas susceptible to bursting as a result of dangerous overpressures [08].

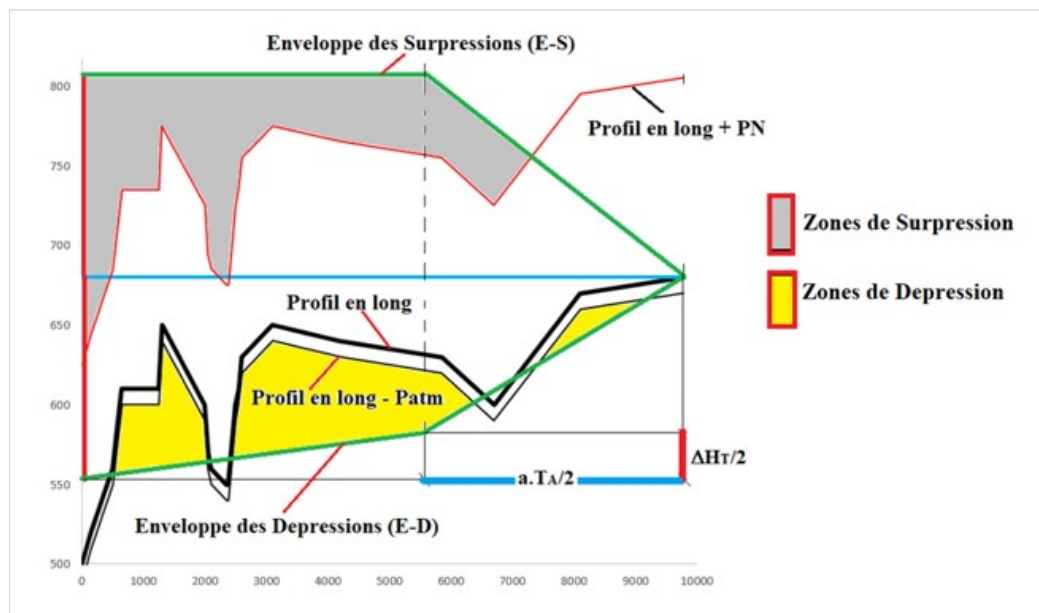


Figure V.3: The ENVELOPE method for pump flow analysis in sudden water hammer case [08]

V.12 Application of the envelope method

Stretch number 01: PS1.1- CMC

Tableau V.11: Water hammer analysis for the first stretch PS1.1 - CMC

x	y	Pro - Patm	PN16	PN20	PN25	PN40
0	325,4	315,52	485,4	525,4	575,4	725,4
292	301	291,12	461	501	551	701
584	320	310,12	480	520	570	720
788	302	292,12	462	502	552	702
983	327	317,12	487	527	577	727
1080	317	307,12	477	517	567	717
1700	460	450,12	620	660	710	860
Enveloppe - surpression				Enveloppe - dépression		
0	608,91			0	311,09	
696,82	608,91			696,82	313,015	
1700	460			1700	460	

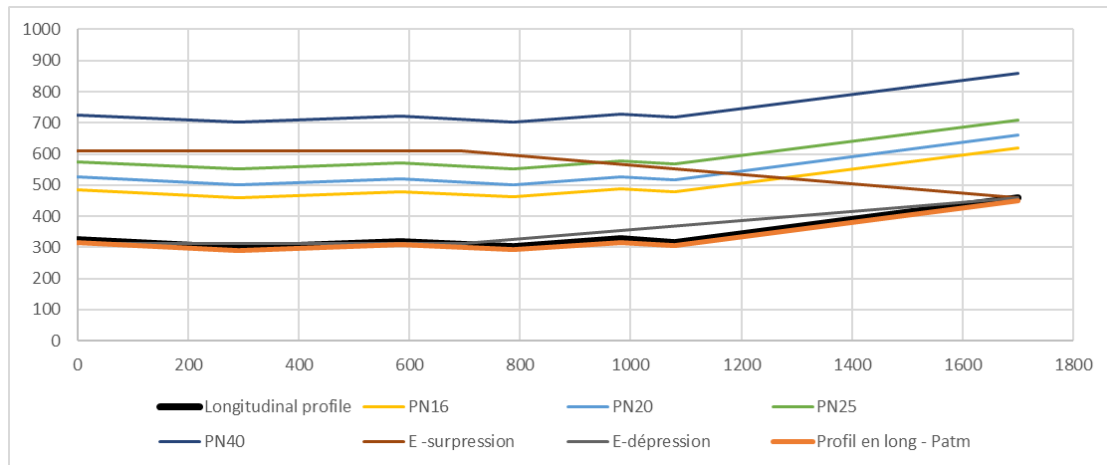


Figure V.4: Graph stretch PS1.1 - CMC

Discussion:

The previous graph shows that the NP40 curve is above the overpressure envelope, indicating that there is no risk of water hammer phenomenon. In other hand, the curve of the pipeline profile is below the depression envelope so there is no risk of water hammer phenomenon.

Stretch number 02: PSB,1- PSB,2

Tableau V.12: Water hammer analysis for the stretch 02 PSB.1-PSB.2

x	y	Pro-Patm	PN16	PN20	PN25	PN40
0	450	440,24	610	650	700	850
597	525	515,24	685	725	775	925
731	518	508,24	678	718	768	918
906	555	545,24	715	755	805	955
1100	510	500,24	670	710	760	910
4430	629,3	619,54	789,3	829,3	879,3	1029,3
Envolpe - surpression			Envolpe - dépression			
0	768,47		0	490,13		
2669,5	768,47		2669,5	509,195		
4430	629,3		4430	629,3		

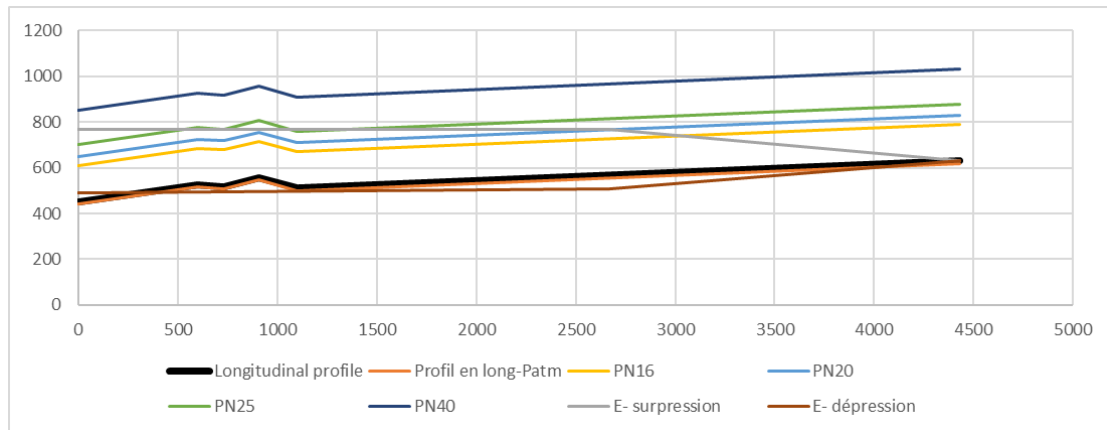


Figure V.5: Graph stretch PSB.1-PSB.2

Discussion:

This graph shows that the NP40 curve is above the overpressure envelope, indicating that there is no risk of water hammer phenomenon. In the other side, the curve of the pipeline profile is below than the depression envelope from 0m to 300m, so there is no risk of water hammer phenomenon. Then, from 300 to 4430m the curve of pipeline profile is above the depression envelope. So, there will be water hammer phenomenon.

Stretch number 03: PSB.2 - R Bouhamza

Tableau V.13: Water hammer anlysis for the stretch 03 PSB.2 - R Bouhamza

x	y	Pro-Patm	PN16	PN20	PN25	PN40
0	629,3	619,72	789,3	829,3	879,3	1029,3
189	646	636,42	806	846	896	1046
286	623	613,42	783	823	873	1023
448	675	665,42	835	875	925	1075
518	659	649,42	819	859	909	1059
793	718	708,42	878	918	968	1118
1106	726	716,42	886	926	976	1126
1213	743	733,42	903	943	993	1143
Enveloppe - surpression		Enveloppe - dépression				
0	882,17	0	603,83			
821,39	882,17	821,39	608,01			
1213	743	1213	743			

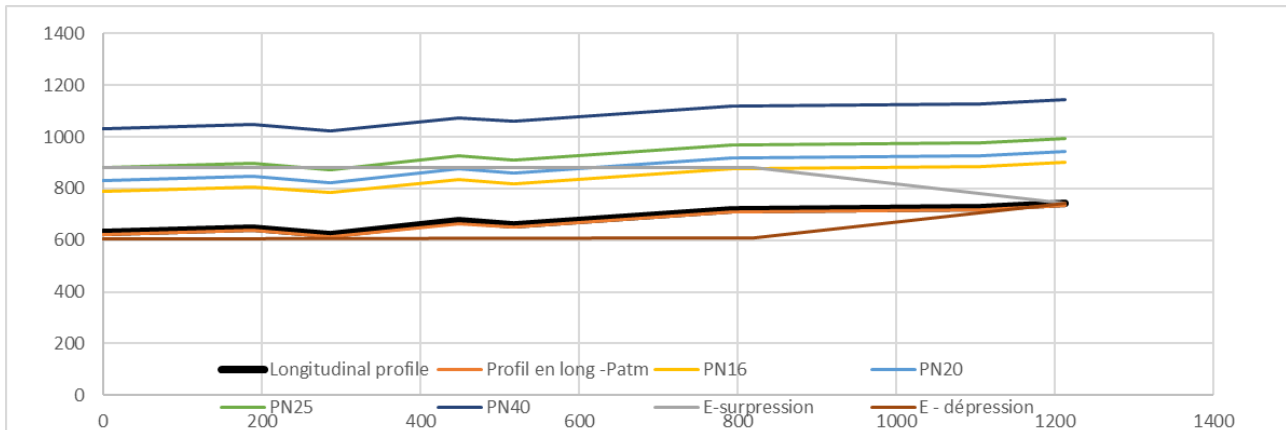


Figure V.6: Graph stretch PSB.2 - R Bouhamza

Discussion:

This graph shows that the PN40 curve is above the overpressure envelope, indicating that there is no risk of water hammer phenomenon. And, the curve of pipeline profile is greater than the depression envelope. Thus, there is a risk of water hammer phenomenon.

Stretch number 04: PS2.1 -PS2.2

Tableau V.14: Water hammer analysis for the stretch 04 PS2.1- PS2.2

x	y	Pro-Patm	PN16	PN20	PN25	PN40
0	443,18	433,41	603,18	643,18	693,18	843,18
898	630	620,23	790	830	880	1030
1390	588	578,23	748	788	838	988
2056	642,18	632,41	802,18	842,18	892,18	1042,18
Enveloppe - surpression			Enveloppe - dépression			
0	772,47		0	511,89		
514,24	772,42		514,24	515,315		
2056	642,18		2056	642,18		

Discussion:

This graph shows that the NP40 curve is above the over pressure envelope, indicating that there is no risk of water hammer phenomenon. In the other side, the profile of the pipeline is greater than the depression envelope from 300 m to 2056 m. Thus, there is a risk of water hammer phenomenon.

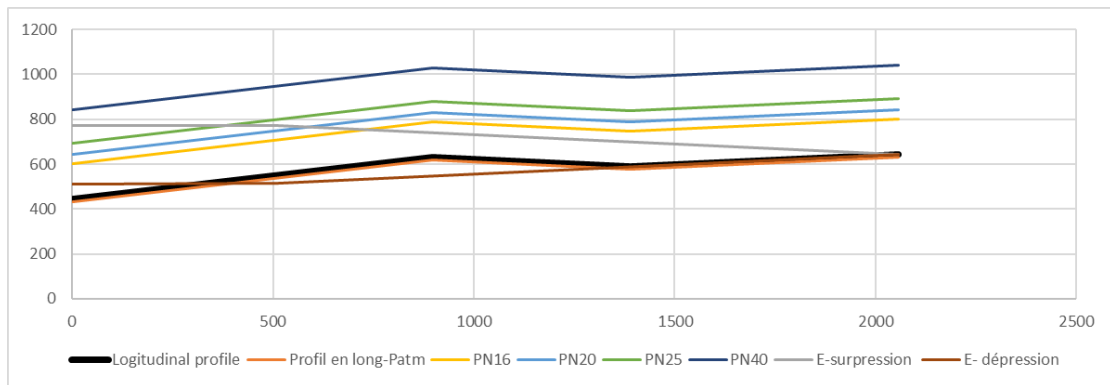


Figure V.7: Graph stretch PS2.1-PS2.2

Stretch number 05: PS2.2 – PS2.3

Tableau V.15: Water hammer analysis for the stretch 05 PS2.2-PS2.3

x	y	Pro-Patm	PN16	PN20	PN25	PN40
0	642,18	632,61	802,18	842,18	892,18	1042,18
603	791	781,43	951	991	1041	1191
895	875,18	865,61	1035,18	1075,18	1125,18	1275,18
Enveloppe - surpression			Enveloppe - dépression			
0	1005,47		0	744,89		
194,71	1005,47		194,71	746,38		
895	875,18		895	875,18		

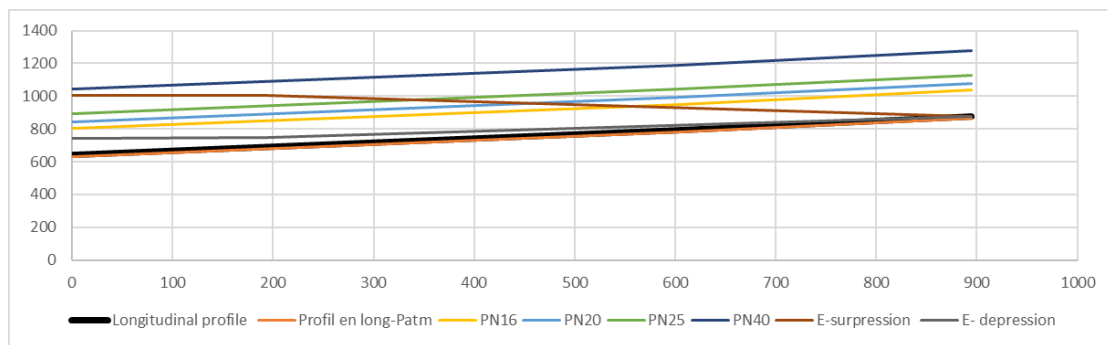


Figure V.8: Graph stretch PS2.2-PS2.3

Discussion:

This graph shows that the NP40 curve is above the overpressure envelope, indicating that there is no risk of water hammer phenomenon. In the other hand, the

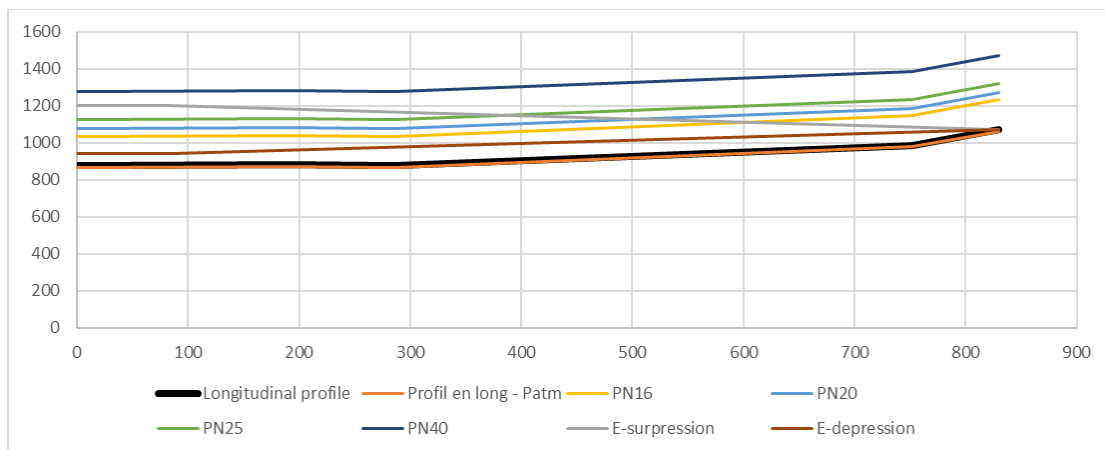
depression envelope is greater than the pipeline profile. Therefore, there is no risk of water hammer phenomenon.

Stretch number 06: PS2.3 – R-Elmain

Tableau V.16: Water hammer analysis for the stretch 06 PS2.3- R Elmain

x	y	Pro-Patm	PN16	PN20	PN25	PN40
0	875,18	865,85	1035,18	1075,18	1125,18	1275,18
192	881	871,67	1041	1081	1131	1281
285	875	865,67	1035	1075	1125	1275
752	986	976,67	1146	1186	1236	1386
830	1072	1062,67	1232	1272	1322	1472

Envolpe - surpression		Envolpe - dépression	
0	1202,29	0	941,71
84,87	1202,29	84,87	943,09
830	1072	830	1072



FigureV.9: Graph stretch PS2.3- R Elmain

Discussion:

Firstly, the previous graph shows that the NP40 curve is above the overpressure envelope, indicating that there is no risk of water hammer phenomenon. Secondly, the pipeline profile is below than the depression envelope. Thus, there is no risk of water hammer phenomenon.

Stretch number 07: PS3.1 – PS3.2

Tableau V.17: Water hammer analysis for the stretch 07 PS3.1 -PS3.2

x	y	Pro-Patm	PN16	PN20	PN25	PN40
0	998,2	988,99	1158,2	1198,2	1248,2	1398,2
40,9	1052	1042,79	1212	1252	1302	1452
136	1049	1039,79	1209	1249	1299	1449
304,1	1065	1055,79	1225	1265	1315	1465
813	1145	1135,79	1305	1345	1395	1545
876	1135	1125,79	1295	1335	1385	1535
1130	1173	1163,79	1333	1373	1423	1573
1440	1126	1116,79	1286	1326	1376	1526
2250	1176	1166,79	1336	1376	1426	1576
2350	1156	1146,79	1316	1356	1406	1556
2500	1165	1155,79	1325	1365	1415	1565
2780	1140	1130,79	1300	1340	1390	1540
2970	1151	1141,79	1311	1351	1401	1551
4073	1208,18	1198,97	1368,18	1408,18	1458,18	1608,18

Enveloppe - surpression		Enveloppe - dépression	
0	1338,47	0	1077,89
733,91	1338,47	733,91	1085,59
4073	1208,18	4073	1208,18

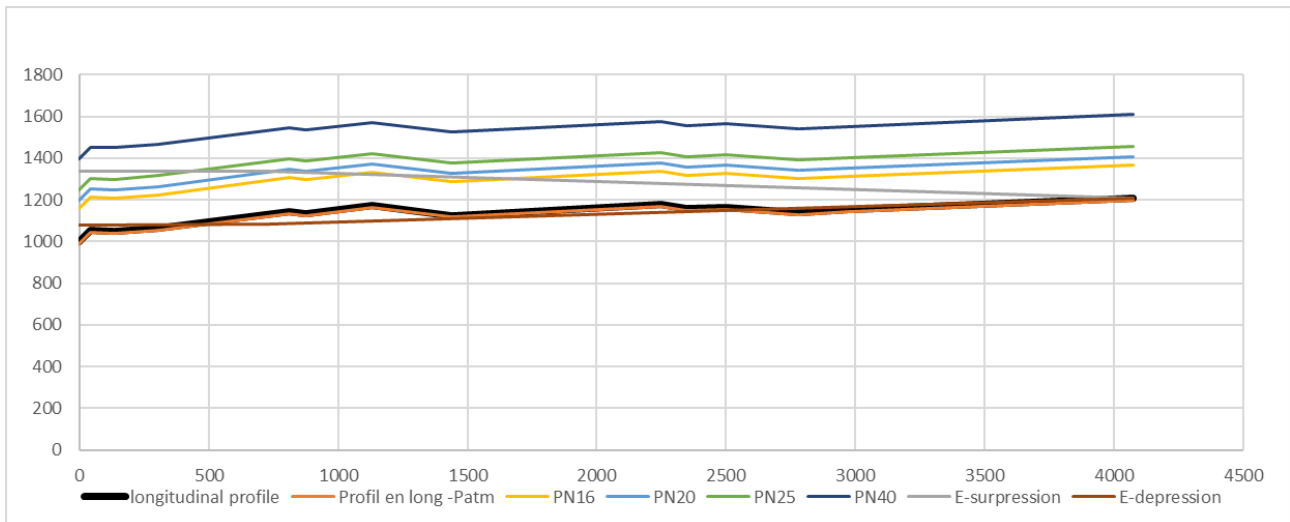


Figure V.10: Graph stretch PS3.1 -PS3.2

Discussion:

Firstly, the previous graph shows that the NP40 curve is above the overpressuenvelope, indicating that there is no risk of water hammer phenomenon. Secondly, the profile of the pipeline is above than the depression envelope from 400 m to 2700 m. In conclusion, there is risk of water hammer phenomenon in this part of the pipeline.

Stretch number 07: PS3.2– R-DJAAFRA

Tableau V.18: Water hammer analysis for the stretch 08 PS3.2- R Djaafra

x	y	Pro-Patm	PN16	PN20	PN25	PN40
0	1208,18	1199,18	1368,18	1408,18	1458,18	1608,18
749	1299	1290	1459	1499	1549	1699
1550	1237	1228	1397	1437	1487	1637
2070	1255	1246	1415	1455	1505	1655
3020	1179	1170	1339	1379	1429	1579
4470	1329	1320	1489	1529	1579	1729
4752	1383,21	1374,21	1543,21	1583,21	1633,21	1783,21
Envloppe - surpression		Envloppe - dépression				
0	1500,26	0	1266,16			
2691,01	1500,26	2691,01	1273,56			
4752	1383,21	4752	1383,21			

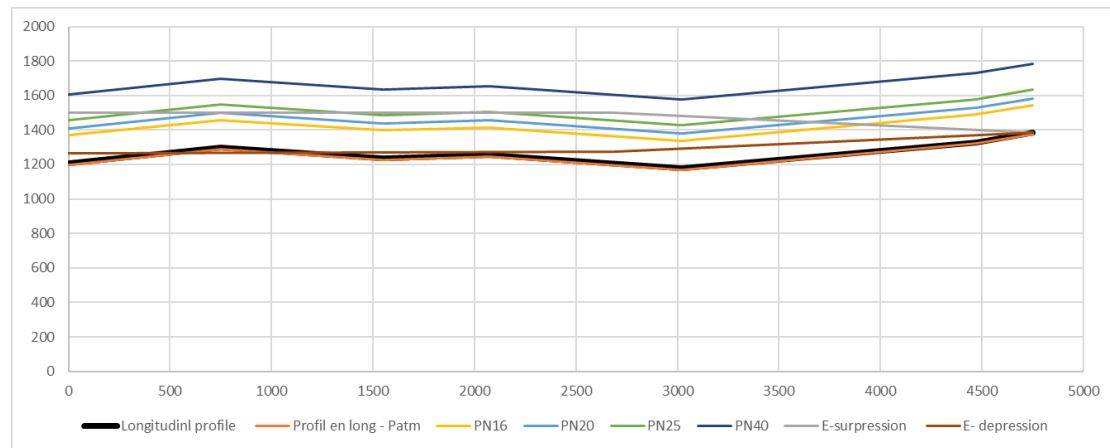


Figure V.11: Graph stretch PS3.2 -R Djaafra

Discussion:

This graph shows that the NP40 curve is above the overpressure envelope, indicating that there is no risk of water hammer phenomenon. Secondly, the profile of

the pipeline is greater than the depression envelope from 600 m to 1100 m so there is risk of water hammer phenomenon in this part of the pipeline.

V.13 Means of protection against water hammer

When water hammer is dangerous due to overpressure alone, the following measures can be used [08]:

- Selecting a higher NP rating for the pipeline and hydraulic components;
- Sizing relief valves;
- Sizing dual-function air valves with high air Pumped capacity;
- Sizing an anti-water hammer tanks.

When water hammer is dangerous due to depression alone, the following measures can be taken [08]:

- Calculating the allowable depression in the pipeline and conducting necessary checks;
- Sizing inertia wheels;
- Sizing dual-function air valves with high air intake capacity;
- Sizing an anti-water hammer tanks.

When water hammer is dangerous simultaneously due to overpressure and depression, the following measures can be taken [08]:

- Selecting a higher PN rating for the pipeline and hydraulic components + calculating the allowable depression in the pipeline;
- Sizing triple-function air valves;
- Sizing an anti-water hammer tanks;
- Sizing a hybrid protection system composed of relief valves and dual-function air valves with high air intake capacity.

V.14 Conclusion

An analysis regarding the choice of the pump was conducted in this chapter. Firstly, a general idea about pumps was presented. Then, the decision was made to select the LOWARA and GOULDS brands from Xylem. Also, Parallel and serial

coupling of pumps was established for each case as required, and the condition of no-cavitation was checked for each pumping station. Finally, calculations were conducted to determine surges and depressions in the Pumped pipes, revealing that maximal pressure (P_{max}) values indicate all sections of the pipelines are under nominal pressure. Consequently, a water hammer analysis was performed on the Pumped pipes using the envelope method.

In the next chapter, we will talk about tanks and their sizing.

Chapter VI

Sizing of Tanks

VI.1 Introduction

The municipalities of the two provinces of Bejaia and Bordj Bou Arreridj will be supplied with drinking water from the Tichy-Haf dam. The purpose of this chapter is to determine the necessary storage volumes for the reservoirs in each municipality to ensure optimal functioning of the distribution networks.

VI.2 Tank presentation

VI.2.1 Definition

Water tanks are generally necessary to adequately supply a community with drinking water. They are primarily dictated by the difference between the water intake or Pumped flow rate and the water consumption rate by the community (which varies depending on the time of day)[13].

VI.2.2 Types of reservoirs

- ✓ Reservoirs are classified based on their position relative to the ground [13]:
 - Underground reservoirs;
 - Semi-buried reservoirs;
 - Elevated reservoirs.
- ✓ In terms of their connection to the supply network, they can also be grouped into two types [13]:
 - Passage reservoirs (located between the intake and the water distribution network);
 - Balancing reservoirs (located at the end of the distribution network).

VI.2.3 Function and utility of reservoirs

The presence of a reservoir between water abstraction, and the distribution network serves several functions [13]:

- ✗ Flow regulation;
- ✗ Pressure regulation;
- ✗ Safety element;

- x Ensuring firefighting reserve;
- x Surge protection;
- x Acting as a relay (buffer reservoir);
- x Ensuring continuity of distribution during pump shutdown.

VI.2.4 Site Selection for Water Supply Reservoirs

The location of water supply reservoirs is determined based on the topography of the study area, taking into account the following considerations [13]:

- x The distribution network must be gravity-fed, so the reservoir should be built at a higher elevation than the community it serves;
- x When multiple reservoirs are necessary, they should preferably be located either at the end of the distribution network or near a significant consumption center;
- x For elevated reservoirs (water towers), the site selection should follow a simple rule: find the minimum elevation ensuring sufficient service to the highest houses.

VI.3 Evaluation of hourly consumption based on the number of inhabitants

“The hourly flow of a city varies according to its size. The variation in daily hourly flows is represented according to the number of inhabitants in the table below”[03].

Tableau VI.1: Adjustment of hourly flow rates based on the number of inhabitants [14]

Heures	Nombre d'habitants				
	(h)	< 10000	10001 à 50000	50001 à 100000	> 100000
0-1	01	1.5	03	3.35	0.75
1-2	01	1.5	3.2	3.25	0.75
2-3	01	1.5	2.5	3.3	01
3-4	01	1.5	2.6	3.2	01
4-5	02	2.5	3.5	3.25	03
5-6	03	3.5	4.1	3.4	5.5
6-7	05	4.5	4.5	3.85	5.5
7-8	6.5	5.5	4.9	4.45	5.5
8-9	6.5	6.25	4.9	5.2	3.5
9-10	5.5	6.25	4.6	5.05	3.5
10-11	4.5	6.25	4.8	4.85	06
11-12	5.5	6.25	4.7	4.6	8.5
12-13	07	05	4.4	4.6	8.5
13-14	07	05	4.1	4.55	06
14-15	5.5	5.5	4.2	4.75	05
15-16	4.5	06	4.4	4.7	05
16-17	05	06	4.3	4.65	3.5
17-18	6.5	5.5	4.1	4.35	3.5
18-19	6.5	05	4.5	4.4	06
19-20	5.0	4.5	4.5	4.3	06
20-21	4.5	04	4.5	4.3	06
21-22	03	03	4.8	3.75	03
22-23	02	02	4.6	3.75	02
23-24	01	1.5	3.3	3.7	01
Somme	100	100	100	100	100

VI.4 Tank capacity

Most often, the reservoir capacity is calculated to meet daily variations in consumption flow, taking into account, of course the day of highest consumption. The reservoir capacity is determined based on the input flow rate and the output flow rate, possibly increased by the fire reserve. Two methods can be applied to determine it [14]:

- The analytical method;
- The graphical method.

VI.4.1 Analytical Method

The analytical method is based on the maximum daily consumption rate and the pump operation time. The total volume V_t is determined by the following formula [14]:

$$V_t = V_r + V_{inc} \quad (VI.1)$$

Where, the regulating volume V_r is calculated using the following formula [14]:

$$V_r = \frac{Q_{max\ day} * P_{max\ \%}}{100} \quad (VI.2)$$

and,

$$P_{max} = V + (\%) + V - \% \quad (VI.3)$$

Given that:

- $V -$: The largest negative value of the difference between the incoming volume and the outgoing volume;
- $V +$: The largest positive value of the difference between the incoming volume and the outgoing volume;
- $Q_{max.day}$: Maximum daily inflow into the reservoir (m^3/day);
- V_{inc} : Fire volume $V_{inc} = 120\ m^3/s$

VI.5 Sizing of tanks

If the tank is circular (assuming a height $h = 4\ m$), then:

$$D(m) = \left(\frac{5 * Vt}{h * \pi} \right)^{(1/2)} \quad (VI.4)$$

Remark:

The future tanks will be of the semi-buried type, circular in shape for low-capacity reservoirs, and rectangular or square for larger capacities. A height of 4 meters has been chosen for each tank.

VI.5.1 Tank sizing for CMC

The arrival at the reservoir is done by pumping, where the pumping time $T_p = 22$ hours.

Tableau VI.2: Tank sizing for CMC

T(h)		Contribution %	Distribution departure (Gravitational) %	Volume for 2786.83 (m ³ /day)	Distribution departure (Gravitational) %	Volume for 57213.17 m ³ /day	Sum (m ³ /day)	Sum %	Water distributed from the reservoir %	Water stored in the reservoir %	Remaining %
0	1	4.55	4.16	115.93	4.16	2380.07	2496.00	4.17		0.38	0.38
1	2	4.55	4.16	115.93	4.16	2380.07	2496.00	4.17		0.38	0.77
2	3	4.55	4.16	115.93	4.16	2380.07	2496.00	4.17		0.38	1.15
3	4	4.55	4.16	115.93	4.16	2380.07	2496.00	4.17		0.38	1.54
4	5	4.55	4.16	115.93	4.16	2380.07	2496.00	4.17		0.38	1.92
5	6	4.55	4.17	116.21	4.17	2385.79	2502.00	4.17		0.38	2.30
6	7	4.55	4.17	116.21	4.17	2385.79	2502.00	4.17		0.38	2.69
7	8	4.55	4.17	116.21	4.17	2385.79	2502.00	4.17		0.38	3.07
8	9	4.55	4.17	116.21	4.17	2385.79	2502.00	4.17		0.38	3.46
9	10	4.55	4.17	116.21	4.17	2385.79	2502.00	4.17		0.38	3.84
10	11	4.55	4.17	116.21	4.17	2385.79	2502.00	4.17		0.38	4.22
11	12	4.55	4.17	116.21	4.17	2385.79	2502.00	4.17		0.38	4.61
12	13	4.55	4.17	116.21	4.17	2385.79	2502.00	4.17		0.37	4.98
13	14	4.55	4.17	116.21	4.17	2385.79	2502.00	4.17		0.37	5.36
14	15	4.55	4.17	116.21	4.17	2385.79	2502.00	4.17		0.37	5.73

15	16	4.55	4.17	116.21	4.17	2385.79	2502.00	4.17		0.37	6.10
16	17	4.55	4.17	116.21	4.17	2385.79	2502.00	4.17		0.37	6.48
17	18	4.55	4.17	116.21	4.17	2385.79	2502.00	4.166		0.37	6.85
18	19	4.55	4.17	116.21	4.17	2385.79	2502.00	4.166		0.37	7.23
19	20	4.55	4.17	116.21	4.17	2385.79	2502.00	4.166	0.37		7.60
20	21	4.55	4.17	116.21	4.17	2385.79	2502.00	4.170	0.37		7.97
21	22	0	4.16	115.93	4.16	2380.07	2496.00	4.170	-4.17		3.80
22	23	0	4.16	115.93	4.16	2380.07	2496.00	4.170	-4.17		-0.37
23	24	4.55	4.16	115.93	4.16	2380.07	2496.00	4.170		0.37	0.00
Total		100	100	2786.83	100	5721.17	60000	100	-7.60	7.60	

Pmax (%)	9.08
Vr	5450.40
Vr+ Vinc = Vt	5570.40
Vt (m³)	6000
h (m)	4
S (m²)	1500
D (m)	43.71

Remark: It is preferred to build 2 tanks of 3000 m³ each, with a diameter of 30.9 meters.

VI.5.2 Tank sizing for the municipality of Bouhamza, Bejaia province

Tableau VI.3: Tank sizing for the municipality of Bouhamza

T(h)		Hourly consumption of (Q max/day)	Contribution %	Water distributed from the reservoir %	Water stored in the reservoir %	Remaining %
0	1	1.5	5		3.5	3.5
1	2	1.5	5		3.5	7
2	3	1.5	5		3.5	10.5
3	4	1.5	5		3.5	14
4	5	2.5	5		2.5	16.5
5	6	3.5	5		1.5	18
6	7	4.5	5		0.5	18.5
7	8	5.5	5	-0.5		18
8	9	6.25	5	-1.25		16.75
9	10	6.25	5	-1.25		15.5
10	11	6.25	5	-1.25		14.25
11	12	6.25	5	-1.25		13
12	13	5	5	0		13
13	14	5	5	0		13
14	15	5.5	5	-0.5		12.5
15	16	6	5	-1		11.5
16	17	6	5	-1		10.5
17	18	5.5	5	-0.5		10
18	19	5	5	0		10
19	20	4.5	0	-4.5		5.5
20	21	4	0	-4		1.5
21	22	3	0	-3		-1.5
22	23	2	0	-2		-3.5
23	24	1.5	5		3.5	0
Total		100	100	-22	22	

Pmax (%)	22
Vr	613.10
Vr+ Vinc = Vt	733.10
Vt (m³)	1000
h (m)	4
S (m²)	250
D (m)	17.85

The arrival at the tank is done by pumping, with a pumping time of $T_p = 20h$.

VI.5.3 Tank sizing for the municipality of El main, Bordj Bou-Argeridj province

The arrival at the tank is done by pumping, with a pumping time of $T_p = 20h$.

Tableau VI.4: Tank sizing for the municipality of El main

T(h)		Contribution %	Hourly consumption of Q (max/day)	Volume for 3147.93 (m ³ /day)	Distribution departure (Gravitational) %	Volume for 11560.22 (m ³ /day)	Sum (m ³ /day)	Sum %	Water distributed from the reservoir %	Water stored in the reservoir %	Remaining %
0	1	5	1.5	47.22	4.16	480.91	528.12	3.59		1.41	1.41
1	2	5	1.5	47.22	4.16	480.91	528.12	3.59		1.41	2.82
2	3	5	1.5	47.22	4.16	480.91	528.12	3.59		1.41	4.23
3	4	5	1.5	47.22	4.16	480.91	528.12	3.59		1.41	5.64
4	5	5	2.5	78.70	4.16	480.91	559.60	3.80		1.20	6.83
5	6	5	3.5	110.18	4.17	482.06	592.24	4.03		0.97	7.81
6	7	5	4.5	141.66	4.17	482.06	623.72	4.24		0.76	8.57
7	8	5	5.5	173.14	4.17	482.06	655.20	4.45		0.55	9.11
8	9	5	6.25	196.75	4.17	482.06	678.81	4.62		0.38	9.50
9	10	5	6.25	196.75	4.17	482.06	678.81	4.62		0.38	9.88

10	11	5	6.25	196.75	4.17	482.06	678.81	4.62		0.38	10.27
11	12	5	6.25	196.75	4.17	482.06	678.81	4.62		0.38	10.65
12	13	5	5	157.40	4.17	482.06	639.46	4.35		0.65	11.30
13	14	5	5	157.40	4.17	482.06	639.46	4.35		0.65	11.95
14	15	5	5.5	173.14	4.17	482.06	655.20	4.45		0.55	12.50
15	16	5	6	188.88	4.17	482.06	670.94	4.56		0.44	12.94
16	17	5	6	188.88	4.17	482.06	670.94	4.56		0.44	13.38
17	18	5	5.5	173.14	4.17	482.06	655.20	4.45		0.55	13.92
18	19	5	5	157.40	4.17	482.06	639.46	4.35		0.65	14.57
19	20	0	4.5	141.66	4.17	482.06	623.72	4.24	-4.24		10.33
20	21	0	4	125.92	4.17	482.06	607.98	4.13	-4.13		6.20
21	22	0	3	94.44	4.16	480.91	575.34	3.91	-3.91		2.29
22	23	0	2	62.96	4.16	480.91	543.86	3.70	-3.70		-1.41
23	24	5	1.5	47.22	4.16	480.91	528.12	3.59		1.41	0.00
Total		100	100	3147.93	100	11560.22	14708.15	100	-15.98	15.98	

Pmax(%)	15.98
Vr	2350.90
Vr+ Vinc = Vt	2470.90
Vt (m³)	3000
h (m)	4
S (m²)	750
D (m)	30.91

VI.5.4 Tank sizing for the municipality of Tafreg, Bordj Bou-Argeridj province

The arrival at the reservoir is done by pumping, with a pumping time of $T_p = 20h$.

Tableau VI.5: Tank sizing for the municipality of Tafreg

T(h)		Hourly consumption of (Q max/day)	Contribution %	Water distributed from the reservoir %	Water stored in the reservoir %	Remaining %
0	1	1	5		4	4
1	2	1	5		4	8
2	3	1	5		4	12
3	4	1	5		4	16
4	5	2	5		3	19
5	6	3	5		2	21
6	7	5	5	0		21
7	8	6.5	5	-1.5		19.5
8	9	6.5	5	-1.5		18
9	10	5.5	5	-0.5		17.5
10	11	4.5	5		0.5	18
11	12	5.5	5	-0.5		17.5
12	13	7	5	-2		15.5
13	14	7	5	-2		13.5
14	15	5.5	5	-0.5		13
15	16	4.5	5		0.5	13.5
16	17	5	5	0		13.5
17	18	6.5	5	-1.5		12
18	19	6.5	5	-1.5		10.5
19	20	5	0	-5		5.5
20	21	4.5	0	-4.5		1
21	22	3	0	-3		-2

22	23	2	0	-2		-4
23	24	1	5		4	0
Total		100	100	-26	26	

Pmax (%)	25
Vr	283.03
Vr+ Vinc = Vt	403.03
Vt (m³)	500
h (m)	4
S (m²)	125
D (m)	12.62

VI.5.5 Tank sizing for the municipality of Djaafra, Bordj Bou-Argeridj province

The arrival at the reservoir is done by pumping, with a pumping time of $T_p = 20h$.

Tableau VI.6: Tank sizing for the municipality of Djaafra

T(h)		Contribution %	Hourly consumption of (Q max/day)	Volume for 4115.12 (m ³ /day)	Distribution departure (Gravitational) %	Volume for 6312.98 (m ³ /day)	Sum (m ³ /day)	Sum %	Water distributed from the reservoir %	Water stored in the reservoir %	Remaining %
0	1	5	1.5	61.73	4.16	262.62	324.35	3.11		1.89	1.89
1	2	5	1.5	61.73	4.16	262.62	324.35	3.11		1.89	3.78
2	3	5	1.5	61.73	4.16	262.62	324.35	3.11		1.89	5.67
3	4	5	1.5	61.73	4.16	262.62	324.35	3.11		1.89	7.56
4	5	5	2.5	102.88	4.16	262.62	365.50	3.50		1.50	9.05
5	6	5	3.5	144.03	4.17	263.25	407.28	3.91		1.09	10.15
6	7	5	4.5	185.18	4.17	263.25	448.43	4.30		0.70	10.85
7	8	5	5.5	226.33	4.17	263.25	489.58	4.69		0.31	11.15
8	9	5	6.25	257.20	4.17	263.25	520.45	4.99		0.01	11.16
9	10	5	6.25	257.20	4.17	263.25	520.45	4.99		0.01	11.17
10	11	5	6.25	257.20	4.17	263.25	520.45	4.99		0.01	11.18
11	12	5	6.25	257.20	4.17	263.25	520.45	4.99		0.01	11.19
12	13	5	5	205.76	4.17	263.25	469.01	4.50		0.50	11.69
13	14	5	5	205.76	4.17	263.25	469.01	4.50		0.50	12.19
14	15	5	5.5	226.33	4.17	263.25	489.58	4.69		0.31	12.50
15	16	5	6	246.91	4.17	263.25	510.16	4.89		0.11	12.61

16	17	5	6	246.91	4.17	263.25	510.16	4.89		0.11	12.72
17	18	5	5.5	226.33	4.17	263.25	489.58	4.69		0.31	13.02
18	19	5	5	205.76	4.17	263.25	469.01	4.50		0.50	13.52
19	20	0	4.5	185.18	4.17	263.25	448.43	4.30	-4.30		9.22
20	21	0	4	164.60	4.17	263.25	427.86	4.10	-4.10		5.12
21	22	0	3	123.45	4.16	262.62	386.07	3.70	-3.70		1.42
22	23	0	2	82.30	4.16	262.62	344.92	3.31	-3.31		-1.89
23	24	5	1.5	61.73	4.16	262.62	324.35	3.11		1.89	0.00
Total		100	100	4115.12	100	6312.98	10428.1	100	-15.41	15.41	

Pmax (%)	15.41
Vr	1607.28
Vr+ Vinc = Vt	1727.28
Vt (m³)	2000
h (m)	4
S (m²)	500
D (m)	25.24

VI.5.6 Tank sizing for the municipality of Colla, Bordj Bou-Argeridj province

The water flows into the reservoir due to gravity.

Tableau VI.7: Tank sizing for the municipality of Colla

T(h)		Hourly consumption of (Q max/day)	Contribution %	Water distributed from the reservoir %	Water stored in the reservoir %	Remaining %
0	1	1.5	4.16		2.66	2.66
1	2	1.5	4.16		2.66	5.32
2	3	1.5	4.16		2.66	7.98
3	4	1.5	4.16		2.66	10.64
4	5	2.5	4.16		1.66	12.3
5	6	3.5	4.17		0.67	12.97
6	7	4.5	4.17	-0.33		12.64
7	8	5.5	4.17	-1.33		11.31
8	9	6.25	4.17	-2.08		9.23
9	10	6.25	4.17	-2.08		7.15
10	11	6.25	4.17	-2.08		5.07
11	12	6.25	4.17	-2.08		2.99
12	13	5	4.17	-0.83		2.16
13	14	5	4.17	-0.83		1.33
14	15	5.5	4.17	-1.33		0
15	16	6	4.17	-1.83		-1.83
16	17	6	4.17	-1.83		-3.66
17	18	5.5	4.17	-1.33		-4.99
18	19	5	4.17	-0.83		-5.82
19	20	4.5	4.17	-0.33		-6.15
20	21	4	4.17		0.17	-5.98
21	22	3	4.16		1.16	-4.82
22	23	2	4.16		2.16	-2.66

23	24	1.5	4.16		2.66	0
Total		100	100	-19.12	19.12	

Pmax (%)	19.12
Vr	589.16
Vr+ Vinc = Vt	709.16
Vt (m³)	1000
h (m)	4
S (m²)	250
D (m)	17.85

VI.5.7 Tank sizing for the municipality of Theniet enaser, Bordj Bou-Argeridj province

The water flows into the reservoir due to gravity.

Tableau VI.8: Tank sizing for the municipality of Theniet enaser

T(h)		Hourly consumption of (Q max/day)	Contribution %	Water distributed from the reservoir %	Water stored in the reservoir %	Remaining %
0	1	1	4.16		3.16	3.16
1	2	1	4.16		3.16	6.32
2	3	1	4.16		3.16	9.48
3	4	1	4.16		3.16	12.64
4	5	2	4.16		2.16	14.8
5	6	3	4.17		1.17	15.97
6	7	5	4.17	-0.83		15.14
7	8	6.5	4.17	-2.33		12.81
8	9	6.5	4.17	-2.33		10.48
9	10	5.5	4.17	-1.33		9.15
10	11	4.5	4.17	-0.33		8.82
11	12	5.5	4.17	-1.33		7.49
12	13	7	4.17	-2.83		4.66
13	14	7	4.17	-2.83		1.83
14	15	5.5	4.17	-1.33		0.5
15	16	4.5	4.17	-0.33		0.17
16	17	5	4.17	-0.83		-0.66
17	18	6.5	4.17	-2.33		-2.99
18	19	6.5	4.17	-2.33		-5.32
19	20	5	4.17	-0.83		-6.15
20	21	4.5	4.17	-0.33		-6.48
21	22	3	4.16		1.16	-5.32
22	23	2	4.16		2.16	-3.16
23	24	1	4.16		3.16	0

Total		100	100	-22.45	22.45	
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Pmax (%)	22.45
Vr	725.49
Vr+ Vinc = Vt	845.49
Vt (m³)	1000
h (m)	4
S (m²)	250
D (m)	17.85

VI.6 Conclusion

In this chapter, the sizing and capacity calculations of potable water supply tanks were examined in detail.

Firstly, the current drinking water needs of the population were analyzed, alongside demographic growth forecasts for the coming years (chapter number 03). Next, tank capacity calculations were approached using the analytical method. It was determined that the first tank would have a capacity of 6000 m³, filled by pumping over a duration of 22 hours. However, this volume has been divided into two tanks, each with a capacity of 3000 m³. The tank designated for the municipality of Bouhamza has a capacity of 1000 m³. Regarding the municipalities of the Bordj Bou Arreridj province, their capacities are as follows: El Maine with 3000 m³, Tafreg with 500 m³, Djaafra with 2000 m³, Colla and Theniet Enaser with 1000 m³ each.



Conclusion

Conclusion

Conclusion

Through this work, we have studied the project of transferring drinking water from the Tichy-Haf dam to two provinces which are Béjaia and Bordj-Bou-Arréridj, to meet the needs of 6 municipalities as follows:

- In the province of Bordj Bou Arreridj: Djafraa, Teffreg, Colla, El Main, and Theniet Enaser;
- In the province of Béjaia: Bouhamza.

This study on drinking water supply is projected for the year 2060, by which time the population of the entire study area is expected to reach 4,096,896 inhabitants. Consumption flow rates were assessed, with the total sum of the maximum daily flow rate amounting to 59,670.40 m³/day. We have opted for a value of 60,000 m³/day for the design of the drinking water supply network.

For material choice, HDPE and steel were selected. Then we illustrated the project layout using Google Earth and Epanet softwares. Following this, we conducted a comprehensive study of the water supply network, considering both the gravity-fed and pumped sections. This involved calculating various hydraulic parameters such as diameter, velocity, nominal pressure, total head loss, and total dynamic head for each segment. Additionally, we conducted a basic study on water hammer phenomena.

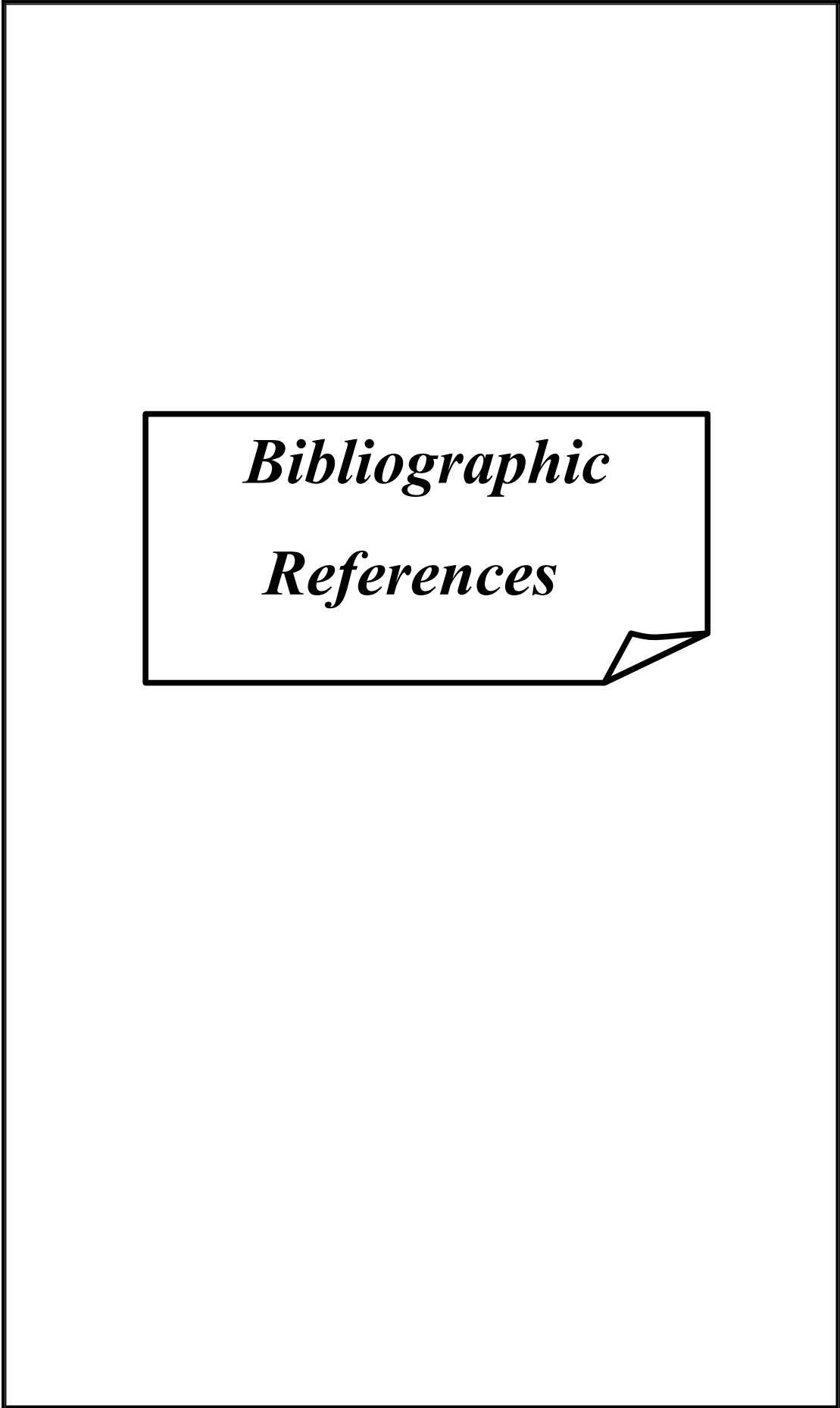
For pump selection, we opted for two brands: LOWARA and GOULDS from Xylem, ensuring that the pumps meet the necessary specifications. These specifications include total dynamic head, efficiency, flow rate, and available net positive suction head (NPSH). The available NPSH should exceed the required NPSH of the pump to prevent cavitation phenomena.

Each municipality necessitates a storage reservoir. We conducted detailed sizing and capacity calculations for potable water supply reservoirs. The initial reservoir, designated as a charging station, would hold a capacity of 6000 m³. Furthermore, the reservoir for the municipality of Bouhamza is sized at 1000 m³. Concerning the municipalities within the Bordj Bou Arreridj province, their respective capacities are as follows: El Main with 3000 m³, Teffreg with 500 m³, Djafraa with

Conclusion

2000 m³, while Colla and Theniet Enaser each require a reservoir with a capacity of 1000 m³.

Ultimately, this internship provided a transition from theory to practical application in the professional world. It enabled me to acquire new knowledge, particularly in using Google Earth and Epanet software. Lastly, we trust that this modest effort will serve as a valuable reference for students.



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Annexes

**Population résidente des ménages ordinaires et collectifs (MOC)
selon la wilaya de résidence et le sexe et le taux d'accroissement annuel moyen (1998-2008)**

Wilayas	الذكور	الإناث	المجموع	معدل النمو	الولاية
	Masculin	Féminin	Total	Taux d'accroissement	
Adrar	203836	195878	399714	2.6	أدرار
Chlef	502470	499618	1002088	1.6	الشلف
Laghouat	232517	223085	455602	3.8	الأغواط
Oum El Bouaghi	314084	307527	621612	1.9	أم البواقي
Batna	565320	554472	1119791	1.6	باتنة
Bejaia	465612	446965	912577	0.6	بجاية
Biskra	365040	356317	721356	2.3	بسكرة
Bechar	135436	134626	270061	1.9	بشار
Blida	509602	493335	1002937	2.5	البليدة
Bouira	353195	342387	695583	1.0	البويرة
Tamanrasset	90820	85816	176637	2.6	تامنغست
Tébessa	326086	322617	648703	1.7	تبسة
Tlemcen	482364	466771	949135	1.2	تلمسان
Tiaret	427358	419465	846823	1.6	تيارت
Tizi Ouzou	564974	562633	1127607	0.2	تيزي وزو
Alger	1499800	1488345	2988145	1.6	الجزائر
Djelfa	562207	529977	1092184	3.3	الجلفة
Jijel	320820	316128	636948	1.1	جيجل
Sétif	756434	733545	1489979	1.3	سطيف
Saida	166717	163925	330641	1.7	سعيدة
Skikda	452392	446288	898680	1.4	سكيكدة
Sidi Bel Abbes	305130	299614	604744	1.4	سيدي بلعباس
Annaba	305030	304469	609499	0.9	عنابة
Guelma	242430	240001	482430	1.2	قالمة

السكان المقيمين من الأسر العادية والجماعية حسب ولاية الإقامة والجنس ومعدل النمو السنوي (1998-2008)
(2008)

Population résidente des ménages ordinaires et collectifs (MOC) selon la wilaya de résidence et le sexe et le taux d'accroissement annuel moyen (1998-2008) (Suite)

Wilaya	الذكور	الإناث	المجموع	معدل النمو	الولاية
	Masculin	Féminin	Total	Taux d'accroissement	
Constantine	470262	468213	938475	1.5	قسنطينة
Médéa	417559	402373	819932	0.2	المدية
Mostaganem	370018	367100	737118	1.6	مستغانم
M'sila	504684	485907	990591	2.1	المسيلة
Mascara	398767	385306	784073	1.5	معسكر
Ouargla	283389	275169	558558	2.3	ورقلة
Oran	728221	725857	1454078	1.9	وهران
El Bayadh	115449	113175	228624	3.1	البيض
Illizi	28244	24088	52333	4.5	إيليزي
Bordj Bou Arreridj	320786	307689	628475	1.3	برج بوعريج
Boumerdes	411650	390433	802083	2.2	بومرداس
El Tarf	203933	204480	408414	1.5	الطارف
Tindouf	24996	24153	49149	6.3	تندوف
Tissemsilt	149434	145042	294476	1.1	تيسمسيلت
El Oued	329604	317944	647548	2.6	الوادي
Khenchela	195502	191182	386683	1.7	خنشلة
Souk Ahras	218911	219216	438127	1.8	سوق أهراس
Tipaza	298885	292125	591010	1.6	تيزازة
Mila	386591	380295	766886	1.3	ميلة
Ain Defla	388776	377236	766013	1.5	عين الدفلى
Naama	98299	94592	192891	4.3	النعامة
Ain Temouchent	187603	183636	371239	1.3	عين تموشنت
Ghardaïa	185209	178390	363598	2.0	غرداية
Relizane	366301	359879	726180	1.3	غليزان
Total	17232747	16847283	34080030	1.6	المجموع

Annex Number 02

Province	Subdivision	Agglomerated population (projection)	Allocation (l/d/inhab)	Qave/d (l/d)	K max/d	Qmax/d (l/d)
Bordj Bou-Arréridj	Colla	7,529	150	1129293	1.45	1,637,474.88
	Djaafra	9,834	150	1475108	1.45	2,138,906.43
	El main	7,669	150	1150319	1.45	1,667,961.92
	Taffreg	2,661	100	266078	1.45	385,812.58
	Ouled Dehmane	20,289	150	3043355	1.45	4,412,865.10
	Thassameurt	5,083	150	762453	1.45	1,105,556.29
	Bordj Zemmoura	12,660	150	1898939	1.60	3,038,301.73
	Theniet Annesr	7,101	150	1065110	1.45	1,544,409.18
Sétif	Ain Legredj	18,035	150	2705287	1.45	3,922,665.61
	Béni Chébana	16,198	150	2429741	1.45	3,523,124.95
	Béni Mohli	10,477	100	1047711	1.45	1,519,181.61
	Béni Ourtilane	13,022	150	1953347	1.45	2,832,352.84
	Harbil	4,519	150	677797	1.45	982,805.84
	Guenzet	4,354	150	653083	1.45	946,970.20
Béjaia	Bouhamza	10,103	150	1515511	1.45	2,197,490.40
						31,855,879.58 l/day
						31,855.88 (m³/day)
		Domestic need (m³/day)	31,855.88			
		Public sector (m³/day)	3938.4			
		Industrial sector (m³/day)	757.39			
		Tertiary sector (m³/day)	1514.77			
		TOTAL	38,066.44	(m³/day)		

Figure 1: Domestic needs for the 15 municipalities projected for the year 2024

Province	Municipality	Agglomerated population (projection)	Allocation (l/d/inhab)	Qave/d (l/d)	K max/d	Qmax/d (l/d)
Bordj Bou-Arréridj	Colla	8,135	150	1220250	1.45	1,769,362.50
	Djaafra	10,627	150	1594050	1.45	2,311,372.50
	El main	8,287	150	1243050	1.45	1,802,422.50
	Taffreg	2,875	100	287500	1.45	416,875.00
	Ouled Dehmane	21,924	150	3288600	1.45	4,768,470.00
	Thassameurt	5,493	150	823950	1.45	1,194,727.50
	Bordj Zemmoura	13,680	150	2052000	1.6	3,283,200.00
	Theniet Annesr	7,673	150	1150950	1.45	1,668,877.50
Setif	Ain Legredj	19,489	150	2923350	1.45	4,238,857.50
	Béni Chébana	17,504	150	2625600	1.45	3,807,120.00
	Béni Mohli	11,321	150	1698150	1.45	2,462,317.50
	Béni Ourtilane	14,072	150	2110800	1.45	3,060,660.00
	Harbil	4,883	150	732450	1.45	1,062,052.50
	Guenzet	4,705	150	705750	1.45	1,023,337.50
Bejaia	Bouhamza	10,498	150	1574700	1.45	2,283,315.00
						35152967.5 l/day
						35,152.97 (m³/day)
		Domestic need (m³/day)	35,152.97			
		Public sector (m³/day)	3938.4			
		Industrial sector (m³/day)	757.39			
		Tertiary sector (m³/day)	1514.77			
		TOTAL	41,363.53	m³/j		

Figure 2: Domestic needs for the 15 municipalities projected for the year 2030

Province	Municipality	Agglomerated population (projection)	Allocation (l/d/inhab)	Qave/d (l/d)	K max/d	Qmax/d (l/d)
Bordj Bou-Arreidj	Colla	9257	150	1388550	1.45	2013397.5
	Djaafra	12092	150	1813800	1.45	2630010
	El main	9429	150	1414350	1.45	2050807.5
	Taffreg	3272	150	490800	1.45	711660
	Ouled Dehmane	24947	150	3742050	1.45	5425972.5
	Thassameurt	6250	150	937500	1.45	1359375
	Bordj Zemmoura	15566	150	2334900	1.6	3735840
	Theniet Annesr	8731	150	1309650	1.45	1898992.5
Setif	Ain Legredj	22175	150	3326250	1.45	4823062.5
	Béni Chébana	19917	150	2987550	1.45	4331947.5
	Béni Mohli	12882	150	1932300	1.45	2801835
	Béni Ourtilane	16012	150	2401800	1.45	3482610
	Harbil	5556	150	833400	1.45	1208430
	Guenzet	5353	150	802950	1.45	1164277.5
Bejaia	Bouhamza	11189	150	1678350	1.45	2433607.5
						40071825 l/day
						40071.825 (m ³ /day)
Domestic need (m³/day)		40,071.83				
Public sector (m³/day)		3938.4				
Industrial sector (m³/day)		757.39				
Tertiary sector (m³/day)		1514.77				
TOTAL		46282.385 (m³/day)				

Figure 3: Domestic needs for the 15 municipalities projected for the year 2040

Province	Municipality	Agglomerated population (projection)	Allocation (l/d/inhab)	Qave/d (l/d)	K max/d	Qmax/d (l/d)	Sparse population projection 2050	Allocation (l/d/inhab)	Qave/d (l/d)	K max/d	Qmax/d (l/d)	Domestic need (m ³ /day)
Bordj Bou-Arreidj	Colla	10533	150	1579950	1.45	2290927.5	715	95	67925	1.45	98491.25	2389418.75
	Djaafra	13759	150	2063850	1.45	2992582.5	1535	95	145825	1.45	211446.25	3204028.75
	El main	10729	150	1609350	1.45	2333557.5	796	95	75620	1.45	109649	2443206.5
	Taffreg	3723	150	558450	1.45	809752.5	498	95	47310	1.45	68599.5	878352
	Ouled Dehmane	28386	150	4257900	1.45	6173955	2908	95	276260	1.45	400577	6574532
	Thassameurt	7112	150	1066800	1.45	1546860	1021	95	96995	1.45	140642.75	1687502.75
	Bordj Zemmoura	17712	150	2656800	1.6	4250880	1552	95	147440	1.6	235904	4486784
	Theniet Annesr	9935	150	1490250	1.45	2160862.5	3158	95	300010	1.45	435014.5	2595877
Setif	Ain Legredj	25233	150	3784950	1.45	5488177.5	2187	95	207765	1.45	301259.25	5789436.75
	Béni Chébana	22663	150	3399450	1.45	4929202.5	2067	95	196365	1.45	284729.25	5213931.75
	Béni Mohli	14658	150	2198700	1.45	3188115	976	95	92720	1.45	134444	3322559
	Béni Ourtilane	18219	150	2732850	1.45	3962632.5	890	95	84550	1.45	122597.5	4085230
	Harbil	6322	150	948300	1.45	1375035	725	95	68875	1.45	99868.75	1474903.75
	Guenzet	6091	150	913650	1.45	1324792.5	1049	95	99655	1.45	144499.75	1469292.25
Bejaia	Bouhamza	11926	150	1788900	1.45	2593905	0	95	0	1.45	0	2593905
						TOTAL	45421237.5	l/day			TOTAL	2787722.75
												48208960.25 l/day
												48,208.96 (m ³ /day)

Horizon of 2050				water needs (m ³ /day)				
Province	Subdivision	Agglomerated population (projection)	Sparse population projection 2050	Domestic need (m ³ /j)	public sector (m ³ /day)	industrial sector (m ³ /day)	tertiary sector (m ³ /day)	TOTAL (m ³ /day)
Bordj Bou-Arreidj	Colla	10533	715	2,389.42	215.92	41.52	83.05	2,729.91
	Djaafra	13759	15087	3,204.03	283.73	54.56	109.13	3,651.45
	El main	10729	11935	2,443.21	219.29	42.17	84.34	2,789.01
	Taffreg	3723	4108	878.35	70.24	13.51	27.01	989.11
	Ouled Dehmane	28386	30739	6,574.53	575.38	110.65	221.3	7,481.86
	Thassameurt	7112	7840	1,687.50	134.91	25.94	51.89	1,900.24
	Bordj Zemmoura	17712	18204	4,486.78	338.65	65.12	130.25	5,020.80
	Theniet Annesr	9935	7903	2,595.88	162.29	31.21	62.42	2,851.80
Setif	Ain Legredj	25233	29137	5,789.44	538.36	103.53	207.06	6,638.39
	Béni Chébana	22663	25421	5,213.93	471.67	90.71	181.41	5,957.72
	Béni Mohli	14658	16240	3,322.56	297.07	57.13	114.26	3,791.02
	Béni Ourtilane	18219	22144	4,085.23	399.62	76.85	153.7	4,715.40
	Harbil	6322	7157	1,474.90	120.6	23.19	46.39	1,665.08
	Guenzet	6091	6264	1,469.29	110.67	21.28	42.57	1,643.81
Bejaia	Bouhamza	11926	0	2,593.91	0	0	0	2,593.91
TOTAL		207001	202894	48,208.96	3938.4	757.37	1514.78	54,419.51

Figure 4: Domestic needs for the 15 municipalities projected for the year 2050

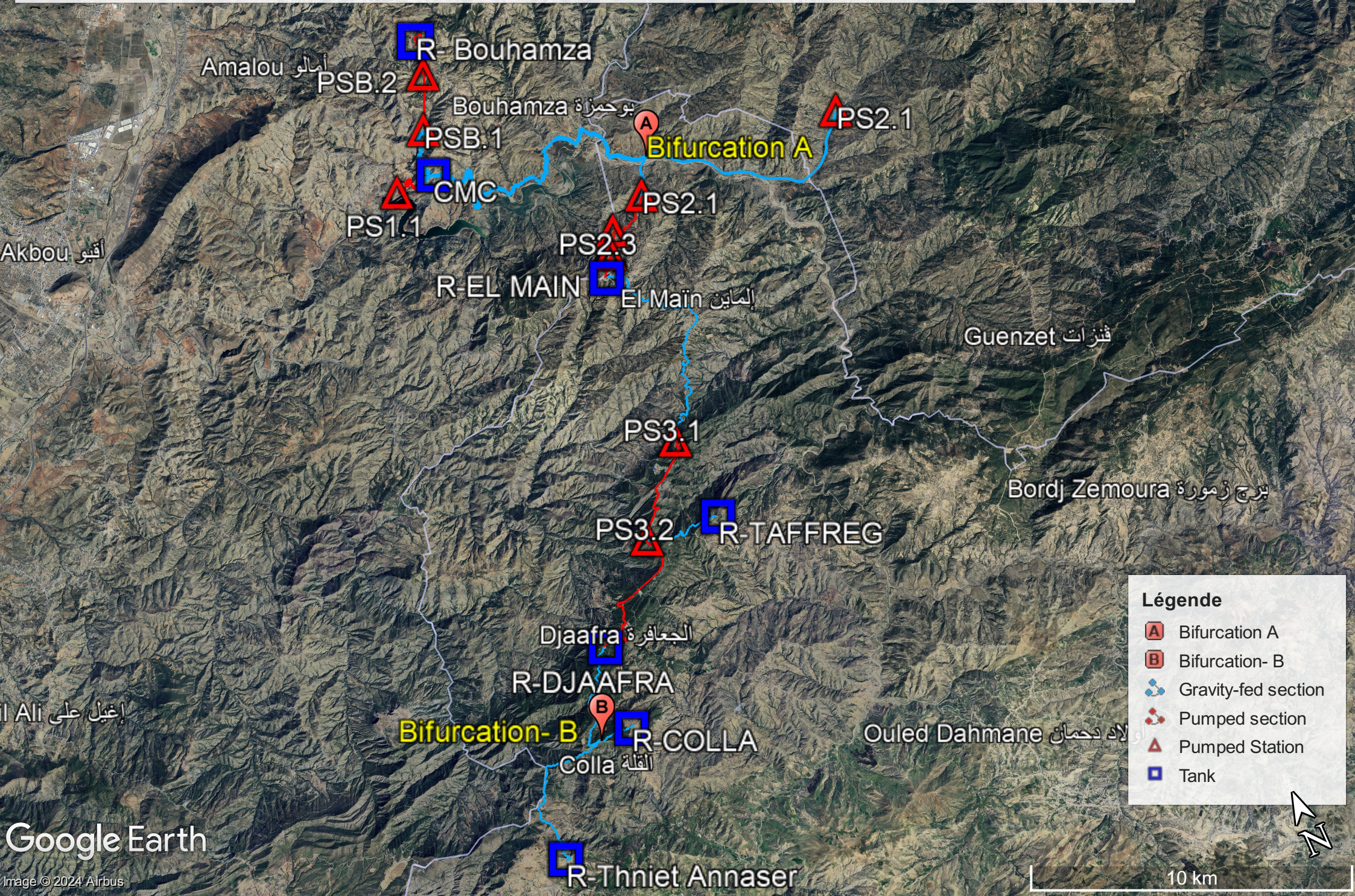
Province	Municipality	Agglomerated population (projection)	Allocation (l/d/inhab)	Qave/d (l/d)	K max/d	Qmax/d (l/d)	Sparse population projection 2050	Allocation (l/d/inhab)	Qave/d (l/d)	K max/d	Qmax/d (l/d)	Domestic need (m ³ /day)
Bordj Bou-Argeridj	Colla	11092	150	1663800	1.45	2,412,510.00	753	95	71535	1.45	103,725.75	2,516,235.75
	Djaafra	14488	150	2173200	1.45	3,151,140.00	1616	95	153520	1.45	222,604.00	3,373,744.00
	El main	11298	150	1694700	1.45	2,457,315.00	838	95	79610	1.45	115,434.50	2,572,749.50
	Taffreg	3920	150	588000	1.45	852,600.00	524	95	49780	1.45	72,181.00	924,781.00
	Ouled Dehmane	29891	150	4483650	1.45	6,501,292.50	3062	95	290890	1.45	421,790.50	6,923,083.00
	Thassameurt	7489	150	1123350	1.45	1,628,857.50	1075	95	102125	1.45	148,081.25	1,776,938.75
	Bordj Zemmoura	18651	150	2797650	1.6	4,476,240.00	1634	95	155230	1.6	248,368.00	4,724,608.00
	Theniet Annesr	10461	150	1569150	1.45	2,275,267.50	3325	95	315875	1.45	458,018.75	2,733,286.25
	Ain Legredj	26571	150	3985650	1.45	5,779,192.50	2303	95	218785	1.45	317,238.25	6,096,430.75
	Béni Chébana	23864	150	3579600	1.45	5,190,420.00	2177	95	206815	1.45	299,881.75	5,490,301.75
Setif	Béni Mohli	15436	150	2315400	1.45	3,357,330.00	1028	95	97660	1.45	141,607.00	3,498,937.00
	Béni Ourtilane	19185	150	2877750	1.45	4,172,737.50	937	95	89015	1.45	129,071.75	4,301,809.25
	Harbil	6657	150	998550	1.45	1,447,897.50	763	95	72485	1.45	105,103.25	1,553,000.75
	Guenzet	6415	150	962250	1.45	1,395,262.50	1105	95	104975	1.45	152,213.75	1,547,476.25
	Bouhamza	12235	150	1835250	1.45	2,661,112.50	0	95	0	1.45	0.00	2,661,112.50
TOTAL						47,759,175.00	l/day	TOTAL			2,935,319.50	50,694,494.50 l/day

50,694,494.50 l/day
50,694.49 (m³/day)

Horizon of 2054				Water needs (m ³ /day)				
Province	Municipality	Agglomerated population (projection)	Sparse population projection 2054	Domestic need (m ³ /day)	public sector (m ³ /day)	industrial sector (m ³ /day)	tertiary sector (m ³ /day)	TOTAL (m ³ /day)
Bordj Bou-Argeridj	Colla	11092	715	2516.23575	215.92	41.52	83.05	2856.72575
	Djaafra	14488	15087	3373.744	283.73	54.56	109.13	3821.164
	El main	11298	11935	2572.7495	219.29	42.17	84.34	2918.5495
	Taffreg	3920	4108	924.781	70.24	13.51	27.01	1035.541
	Ouled Dehmane	29891	30739	6923.083	575.38	110.65	221.3	7830.413
	Thassameurt	7489	7840	1776.93875	134.91	25.94	51.89	1989.67875
	Bordj Zemmoura	18651	18204	4724.608	338.65	65.12	130.25	5258.628
	Theniet Annesr	10461	7903	2733.28625	162.29	31.21	62.42	2989.20625
	Ain Legredj	26571	29137	6096.43075	538.36	103.53	207.06	6945.38075
	Béni Chébana	23864	25421	5490.30175	471.67	90.71	181.41	6234.09175
Setif	Béni Mohli	15436	16240	3498.937	297.07	57.13	114.26	3967.397
	Béni Ourtilane	19185	22144	4301.80925	399.62	76.85	153.7	4931.97925
	Harbil	6657	7157	1553.00075	120.6	23.19	46.39	1743.18075
	Guenzet	6415	6264	1547.47625	110.67	21.28	42.57	1721.99625
Bejaia	Bouhamza	12235	0	2661.1125	0	0	0	2661.1125
TOTAL		217653	202894	50694.4945	3938.4	757.37	1514.78	56905.0445

Figure 5: Domestic needs for the 15 municipalities projected for the year 2054

The project layout from the Tichy-Haf dam to the distribution network tanks



Légende

- Bifurcation A
- Bifurcation-B
- Gravity-fed section
- Pumped section
- Pumped Station
- Tank

Annex 04

Pumped water supply

Stretch 01: PS1.1 -CMC

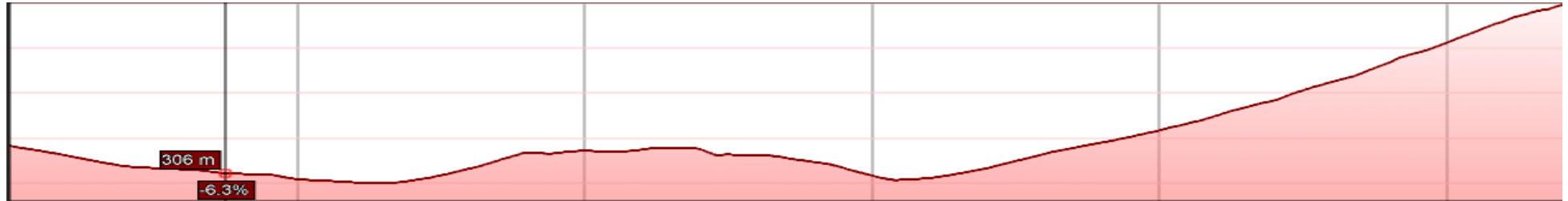


Figure 1: The longitudinal profile of the conduit PS1.1- CMC

Stretch		Cote (m)			roughness of steel K=0,1mm					ΔHs=15% ΔHL					
					flow (m3/h)	length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)
		upstream	downstream	upstream								downstream			
PS1.1	VID 01	325.40	301.00	2,500	292	800	1.51	0.0019756	0.58	0.66	463.85	463.19	162.19	162.85	NP40
VID 01	VNT 01	301.00	320.00	2,500	292	800	1.51	0.0019756	0.58	0.66	463.19	462.52	142.52	143.85	
VNT 01	VID 02	320.00	302.00	2,500	204	800	1.51	0.0019756	0.40	0.46	462.52	462.06	160.06	161.85	
VID 02	VNT 02	302.00	327.00	2,500	195	800	1.51	0.0019756	0.39	0.44	462.06	461.62	134.62	136.85	
VNT 02	VID 03	327.00	317.00	2,500	97	800	1.51	0.0019756	0.19	0.22	461.62	461.40	144.40	146.85	
VID 03	CMC	317.00	460.00	2,500	620	800	1.51	0.0019756	1.22	1.41	461.40	459.99	-0.01	3.85	

Figure 2: The nominal pressure for stretch number 01 PS1.1 -CMC

Stretch 02: PSB.1-PSB.2

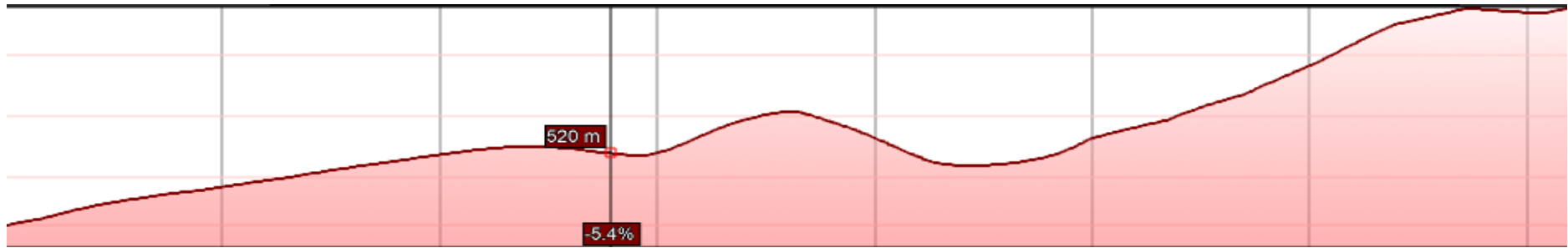


Figure 3: The longitudinal profile of the conduit PSB.1-PSB.2

Stretch		roughness of steel K=0,1mm			ΔHs=15% ΔHL										
		Cote (m)		flow (m3/h)	length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	Nominal Pressure
		upstream	downstream								upstream	downstream			
PSB.1	VNT 01	450.00	525.00	116.12	597	200	1.23	0.0062074	3.71	4.26	659.18	654.92	129.92	134.18	PN40
VNT 01	VID 01	525.00	518.00	116.12	134	200	1.23	0.0062074	0.83	0.96	654.92	653.96	135.96	141.18	
VID 01	VNT 02	518.00	555.00	116.12	175	200	1.23	0.0062074	1.09	1.25	653.96	652.71	97.71	104.18	
VNT 02	VID 02	555.00	510.00	116.12	194	200	1.23	0.0062074	1.20	1.38	652.71	651.33	141.33	149.18	
VID 02	PSB.2	510.00	629.30	116.12	4430	200	1.23	0.0062074	27.50	31.62	651.33	619.70	-9.60	29.88	

Figure 4: The nominal pressure for stretch number 02 PSB.1-PSB.2

Stretch 03: PSB.2- R.Bouhamza

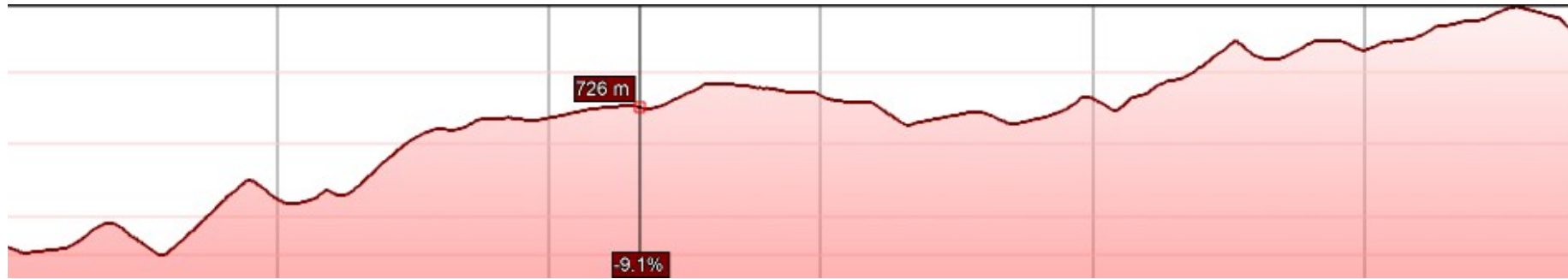


Figure 5: The longitudinal profile of the conduit PSB.2 - R.Bouhamza

Stretch		roughness of steel K=0,1mm			ΔHs=15% ΔHL										
		Cote (m)		flow (m3/h)	length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	Nominal Pressure
		upstream	downstream								upstream	downstream			
PSB.2	VNT 01	629.30	646.00	116.12	189	200	1.23	0.0062074	1.17	1.35	788.36	787.01	141.01	142.36	NP40
VNT 01	VID 01	646.00	623.00	116.12	97	200	1.23	0.0062074	0.60	0.69	787.01	786.32	163.32	165.36	
VID 01	VNT 02	623.00	675.00	116.12	162	200	1.23	0.0062074	1.01	1.16	786.32	785.16	110.16	113.36	
VNT 02	VID 02	675.00	659.00	116.12	70	200	1.23	0.0062074	0.43	0.50	785.16	784.66	125.66	129.36	
VID 02	VNT.03	659.00	718.00	116.12	275	200	1.23	0.0062074	1.71	1.96	784.66	782.70	64.70	70.36	
VNT.03	VID.03	718.00	726.00	116.12	313	200	1.23	0.0062074	1.94	2.23	782.70	780.46	54.46	62.36	
VID.03	R-Bouhamza	726.00	743.00	116.12	107	200	1.23	0.0062074	0.66	0.76	780.46	779.70	36.70	45.36	

Figure 6: The nominal pressure for stretch number 03 SPB.2- R.Bouhamza

Stretch 04: PS2.1 – PS2.2



Figure 7: The longitudinal profile of the conduit PS2.1 - PS2.2

Stretch		Cote (m)			roughness of Steel K=0,1 mm					ΔHs=15% ΔHL					
		upstream	downstream	flow (m3/h)	length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	Nominal Pressure
PS2.1	VNT 01	443.18	630.00	612.84	898	450	1.28	0.0028949	2.60	2.99	649.03	646.04	16.04	19.03	NP40
VNT 01	VID 01	630.00	588.00	612.84	492	450	1.28	0.0028949	1.42	1.64	646.04	644.40	56.40	61.03	
VID 01	PS2.1	588.00	642.18	612.84	666	450	1.28	0.0028949	1.93	2.22	644.40	642.19	0.01	6.85	

Figure 8: The nominal pressure for stretch number 04 PS2.1 - PS2.2

Stretch 05: PS2.2-PS2.3



Figure 9: The longitudinal profile of the conduit PS2.2- PS2.3

Stretch		Cote (m)			roughness of steel K=0,1mm					ΔHs=15% ΔHL					
		upstream	downstream	flow (m3/h)	length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	Nominal Pressure
PS2.2	VNT 01	642.18	791.00	612.84	603	450	1.28	0.0028949	1.75	2.01	878.16	876.15	85.15	87.16	NP40
VNT 01	PS2.3	791.00	875.18	612.84	292	450	1.28	0.0028949	0.85	0.97	876.15	875.18	0.00	2.98	

Figure 10: The nominal pressure for stretch number 05 PS2.2-PS2.3

Stretch 06: PS2.3-R.El main

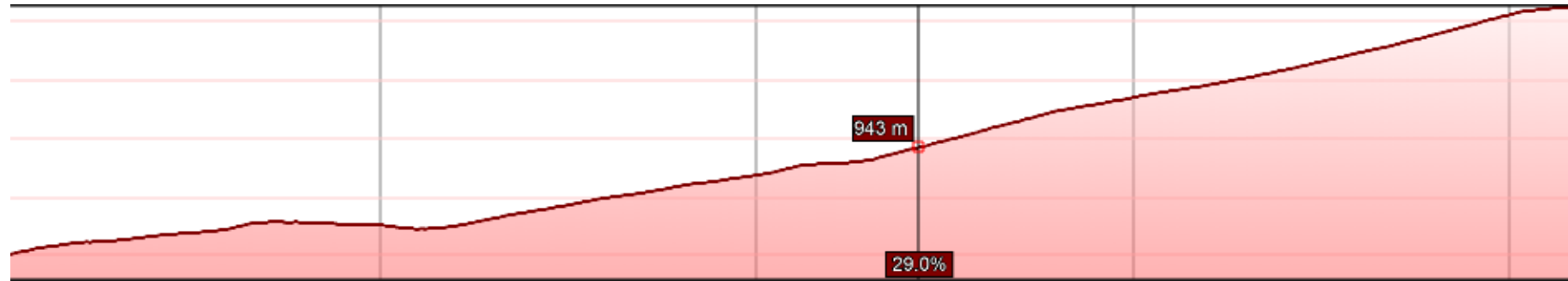


Figure 11: The longitudinal profile of the conduit PS2.3 - R.Elmain

Stretch		Cote (m)			roughness of steel K=0,1mm					ΔHs=15% ΔHL					
		upstream	downstream	flow (m3/h)	length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	Nominal Pressure
											upstream	downstream			
PS2.3	VNT 01	875.18	881.00	612.84	192	450	1.28	0.0028949	0.56	0.64	1074.76	1074.12	193.12	193.76	NP40
VNT 01	VID 01	881.00	875.00	612.84	93	450	1.28	0.0028949	0.27	0.31	1074.12	1073.81	198.81	199.76	
VID 01	VNT 02	875.00	986.00	612.84	467	450	1.28	0.0028949	1.35	1.55	1073.81	1072.26	86.26	88.76	
VNT 02	R-Elmain	986.00	1072.00	612.84	78	450	1.28	0.0028949	0.23	0.26	1072.26	1072.00	0.00	2.76	

Figure 12: The nominal pressure for stretch number 06 PS2.3-R.El main

Stretch 07: PS3.1 -PS3.2

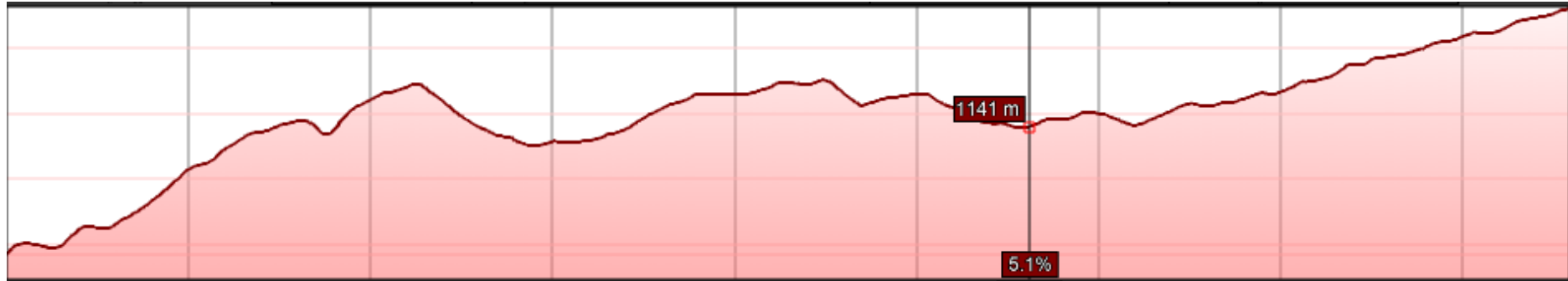


Figure 13: The longitudinal profile of the conduit PS3.1 -PS3.2

Stretch		roughness of steel K=0,1mm			ΔHs=15% ΔHL										
		Cote (m)		flow (m3/h)	length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	Nominal Pressure
		upstream	downstream								upstream	downstream			
PS3.1	VNT 01	998.20	1052.00	481.68	40.9	400	1.28	0.0033403	0.14	0.16	1223.57	1223.41	171.41	171.57	NP40
VNT 01	VID 01	1052.00	1049.00	481.68	95.1	400	1.28	0.0033403	0.32	0.37	1223.41	1223.05	174.05	174.57	
VID 01	VNT 02	1049.00	1065.00	481.68	168.1	400	1.28	0.0033403	0.56	0.65	1223.05	1222.40	157.40	158.57	
VNT 02	VNT.03	1065.00	1145.00	481.68	508.9	400	1.28	0.0033403	1.70	1.95	1222.40	1220.45	75.45	78.57	
VNT.03	VID.02	1145.00	1135.00	481.68	63	400	1.28	0.0033403	0.21	0.24	1220.45	1220.21	85.21	88.57	
VID.02	VNT.04	1135.00	1173.00	481.68	254	400	1.28	0.0033403	0.85	0.98	1220.21	1219.23	46.23	50.57	
VNT.04	VID.03	1173.00	1126.00	481.68	310	400	1.28	0.0033403	1.04	1.19	1219.23	1218.04	92.04	97.57	
VID.03	VNT.05	1126.00	1176.00	481.68	810	400	1.28	0.0033403	2.71	3.11	1218.04	1214.93	38.93	47.57	
VNT.05	VID.04	1176.00	1156.00	481.68	100	400	1.28	0.0033403	0.33	0.38	1214.93	1214.54	58.54	67.57	
VID.04	VNT.06	1156.00	1165.00	481.68	150	400	1.28	0.0033403	0.50	0.58	1214.54	1213.97	48.97	58.57	
VNT.06	VID.05	1165.00	1140.00	481.68	280	400	1.28	0.0033403	0.94	1.08	1213.97	1212.89	72.89	83.57	
VID.05	VNT.07	1140.00	1151.00	481.68	190	400	1.28	0.0033403	0.63	0.73	1212.89	1212.16	61.16	72.57	
VNT.07	PS3.2	1151.00	1208.18	481.68	1103	400	1.28	0.0033403	3.68	4.24	1212.16	1207.92	-0.26	15.39	

Figure 14: The nominal pressure for stretch number 07 SP3.1 -SP3.2

Stretch 08: PS3.2- R.Djaafra

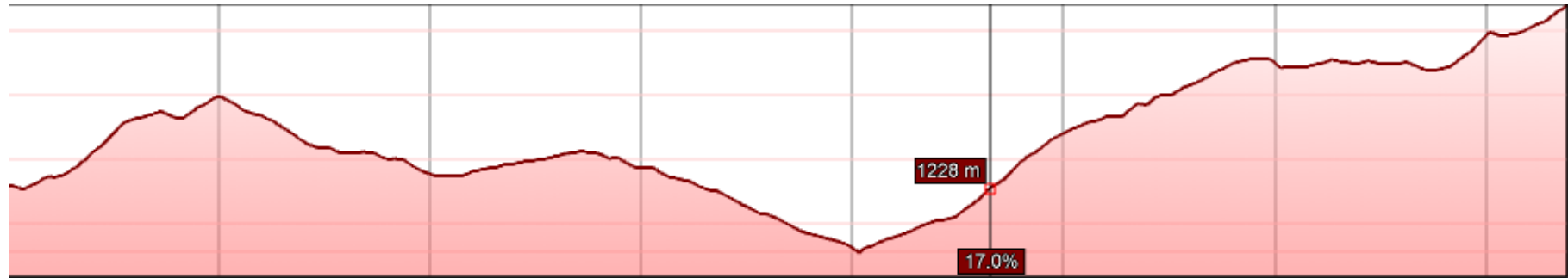


Figure 15: The longitudinal profile of the conduit PS3.2 - R.Djaafra

Stretch		roughness of Steel K=0,1mm			ΔHs=15% ΔHL										
		Cote (m)		flow (m3/h)	length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	Nominal Pressure
		upstream	downstream								upstream	downstream			
PS3.2	VNT 01	1208.18	1299.00	434.50	749	400	1.15	0.0027299	2.04	2.35	1398.01	1395.66	96.66	99.01	NP40
VNT 01	VID 01	1299.00	1237.00	434.50	801	400	1.15	0.0027299	2.19	2.51	1395.66	1393.14	156.14	161.01	
VID 01	VNT 02	1237.00	1255.00	434.50	520	400	1.15	0.0027299	1.42	1.63	1393.14	1391.51	136.51	143.01	
VNT 02	VID 02	1255.00	1179.00	434.50	950	400	1.15	0.0027299	2.59	2.98	1391.51	1388.53	209.53	219.01	
VID 02	VNT 03	1179.00	1329.00	434.50	1450	400	1.15	0.0027299	3.96	4.55	1388.53	1383.98	54.98	69.01	
VNT 03	R-DJAAFRA	1329.00	1383.21	434.50	282	400	1.15	0.0027299	0.77	0.89	1383.98	1383.09	-0.12	14.80	

Figure 16: The nominal pressure for stretch number 08 SP3.2- R.Djaafra

Gravity-fed water supply

Stretch 01: CMC- PSB.1



Figure 17: The longitudinal profile of the conduit CMC - PSB.1

Stretch		Cote (m)			roughness of Steel K=0,1mm					ΔHs=15% ΔHL					
		upstream	downstream	flow (m3/h)	length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	Nominal Pressure
											upstream	downstream			
CMC	VID 01	460.00	395.00	116.12	388	250	0.66	0.0014653	0.57	0.65	460.00	459.35	64.35	65.00	NP10
VID 01	VNT 01	395.00	443.00	116.12	367	250	0.66	0.0014653	0.54	0.62	459.35	458.73	15.73	17.00	
VNT 01	VID 02	443.00	411.00	116.12	208	250	0.66	0.0014653	0.30	0.35	458.73	458.38	47.38	49.00	
VID 02	VNT 02	411.00	461.00	116.12	167	250	0.66	0.0014653	0.24	0.28	458.38	458.10	-2.90	-1.00	
VNT 02	VID 03	461.00	434.00	116.12	150	250	0.66	0.0014653	0.22	0.25	458.10	457.84	23.84	26.00	
VID 03	VNT 03	434.00	458.00	116.12	220	250	0.66	0.0014653	0.32	0.37	457.84	457.47	-0.53	2.00	
VNT 03	VID 04	458.00	441.00	116.12	70	250	0.66	0.0014653	0.10	0.12	457.47	457.35	16.35	19.00	
VID 04	VNT 04	441.00	458.00	116.12	250	250	0.66	0.0014653	0.37	0.42	457.35	456.93	-1.07	2.00	
VNT 04	VID 05	458.00	441.00	116.12	100	250	0.66	0.0014653	0.15	0.17	456.93	456.76	15.76	19.00	
VID 05	VNT 05	441.00	460.00	116.12	80	250	0.66	0.0014653	0.12	0.13	456.76	456.63	-3.37	0.00	
VNT 05	VID 06	460.00	427.00	116.12	300	250	0.66	0.0014653	0.44	0.51	456.63	456.12	29.12	33.00	
VID 06	PSB.1	427.00	450.00	116.12	450	250	0.66	0.0014653	0.66	0.76	456.12	455.37	5.37	10.00	

Figure 18: The nominal pressure for stretch number 01 CMC- PSB.1

Stretch 02: CMC -Bif A

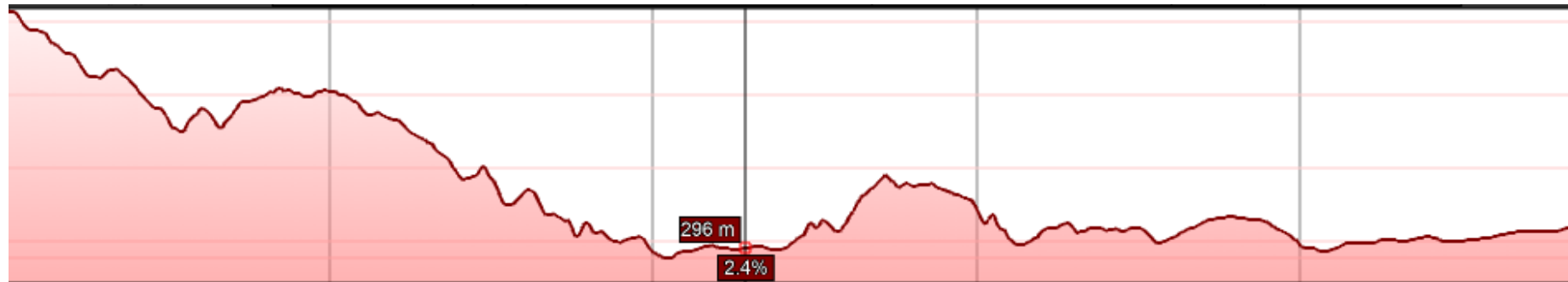


Figure 19: The longitudinal profile of the conduit CMC - Bif A

Stretch		roughness of Steel K=0,1mm			ΔHs=15% ΔHL										
		Cote (m)		flow (m3/h)	length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	Nominal Pressure
		upstream	downstream								upstream	downstream			
CMC	VID 01	460.00	375.00	2,383.88	1060	1000	0.84	0.0004927	0.52	0.60	460.00	459.40	84.40	85.00	NP25
VID 01	VNT 01	375.00	390.00	2,383.88	130	1000	0.84	0.0004927	0.06	0.07	459.40	459.33	69.33	70.00	
VNT 01	VID 02	390.00	377.00	2,383.88	150	1000	0.84	0.0004927	0.07	0.08	459.33	459.24	82.24	83.00	
VID 02	VNT 02	377.00	406.00	2,383.88	470	1000	0.84	0.0004927	0.23	0.27	459.24	458.97	52.97	54.00	
VNT 02	VID 03	406.00	342.00	2,383.88	1440	1000	0.84	0.0004927	0.71	0.82	458.97	458.16	116.16	118.00	
VID 03	VID 04	342.00	288.00	2,383.88	1620	1000	0.84	0.0004927	0.80	0.92	458.16	457.24	169.24	172.00	
VID 04	VNT 03	288.00	344.00	2,383.88	1650	1000	0.84	0.0004927	0.81	0.93	457.24	456.31	112.31	116.00	
VNT 03	VID 05	344.00	298.00	2,383.88	1030	1000	0.84	0.0004927	0.51	0.58	456.31	455.72	157.72	162.00	
VID 05	VNT 04	298.00	358.00	2,383.88	360	1000	0.84	0.0004927	0.18	0.20	455.72	455.52	97.52	102.00	
VNT 04	VID 06	358.00	298.00	2,383.88	1240	1000	0.84	0.0004927	0.61	0.70	455.52	454.82	156.82	162.00	
VID 06	Bif-A	298.00	450.00	2,383.88	2650	1000	0.84	0.0004927	1.31	1.50	454.82	453.31	3.31	10.00	

Figure 20: The nominal pressure for stretch number 02 CMC -Bif A

Stretch 03: Bif A - LOT 03

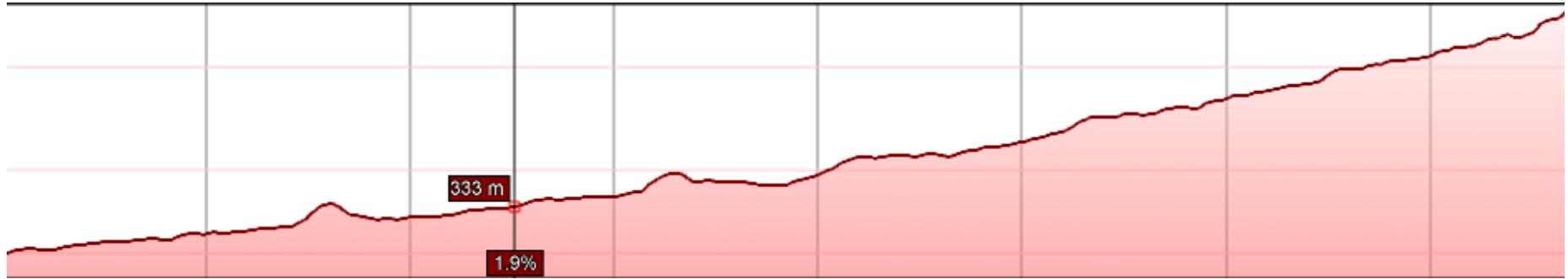


Figure 21: The longitudinal profile of the conduit Bif A - LOT 03

Stretch		roughness of Steel K=0,1mm			ΔHs=15% ΔHL										
		Cote (m)		flow (m3/h)	length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	Nominal Pressure
		upstream	downstream								upstream	downstream			
Bif-A	VNT 01	450.00	334.00	1,771.04	1600	800	0.97	0.0008452	1.35	1.56	450.00	448.44	114.44	116.00	NP16
VNT 01	VID 01	334.00	326.00	1,771.04	330	800	0.97	0.0008452	0.28	0.32	448.44	448.12	122.12	124.00	
VID 01	VNT 02	326.00	349.00	1,771.04	1370	800	0.97	0.0008452	1.16	1.33	448.12	446.79	97.79	101.00	
VNT 02	VNT 02	349.00	343.00	1,771.04	490	800	0.97	0.0008452	0.41	0.48	446.79	446.32	103.32	107.00	
VNT 02	LOT-03	343.00	434.35	1,771.04	3849	800	0.97	0.0008452	3.25	3.74	446.32	442.57	8.22	15.65	

Figure 22: The nominal pressure for stretch number 03 Bif A - LOT 03

Stretch 04: Bif A -PS2.1

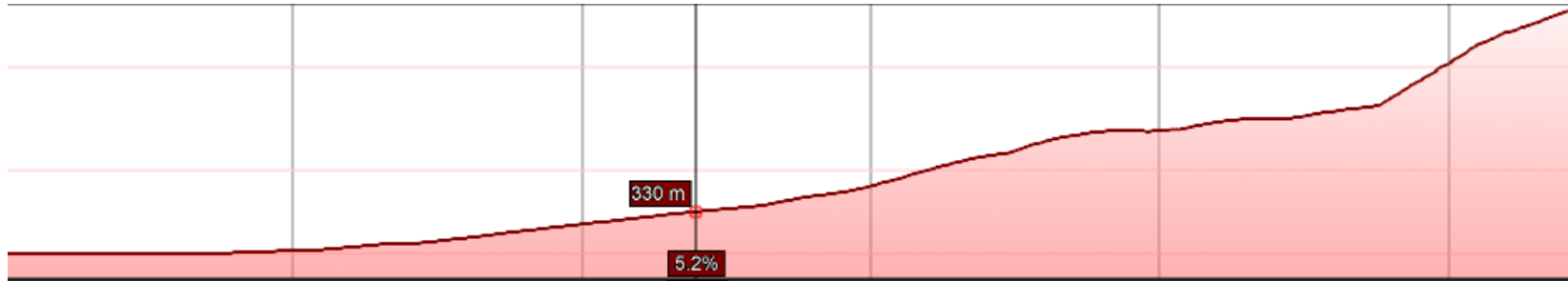


Figure 23: The longitudinal profile of the conduit Bif A - PS2.1

Stretch		Cote (m)			roughness of Steel K=0,1mm					ΔHs=15% ΔHL					
		upstream	downstream	flow (m3/h)	length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	Nominal Pressure
											upstream	downstream			
Bif-A	VID 01	450.00	309.00	612.84	164	450	1.07	0.0017895	0.29	0.34	450.00	449.66	140.66	141.00	NP16
VID 01	VNT 01	309.00	370.00	612.84	798	450	1.07	0.0017895	1.43	1.64	449.66	448.02	78.02	80.00	
VNT 01	PS2.1	370.00	443.18	612.84	378	450	1.07	0.0017895	0.68	0.78	448.02	447.24	4.06	6.82	

Figure 24: The nominal pressure for stretch number 04 Bif A -PS2.1

Stretch 05: R Elmain – PS3.1

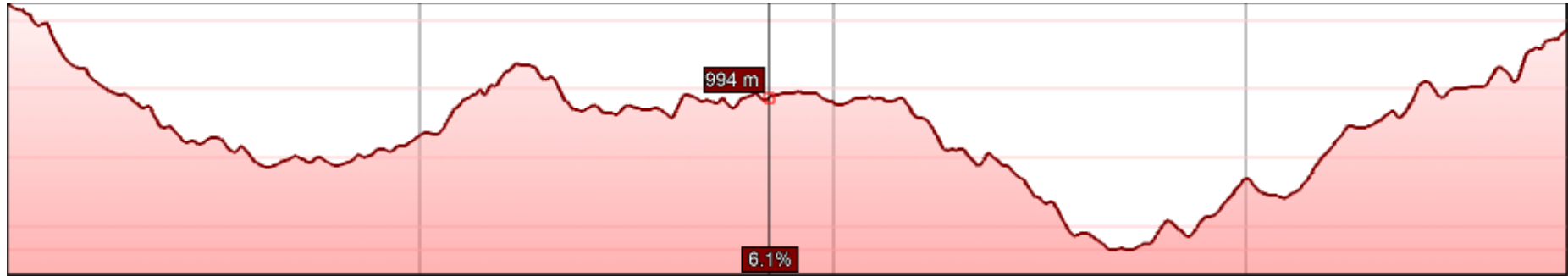


Figure 25: The longitudinal profile of the conduit R Elmain - PS3.1

Stretch		roughness of Steel K=0,1mm			ΔHs=15% ΔHL										
		Cote (m)		flow (m3/h)	length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	Nominal Pressure
		upstream	downstream								upstream	downstream			
R-Elmain	VID 01	1072.00	944.00	481.66	1540	400	1.06	0.0020044	3.09	3.55	1072.00	1068.45	124.45	128.00	NP25
VID 01	VNT 01	944.00	1018.00	481.66	1560	400	1.06	0.0020044	3.13	3.60	1068.45	1064.85	46.85	54.00	
VNT 01	VID 02	1018.00	979.00	481.66	920	400	1.06	0.0020044	1.84	2.12	1064.85	1062.73	83.73	93.00	
VID 02	VNT 02	979.00	994.00	481.66	1390	400	1.06	0.0020044	2.79	3.20	1062.73	1059.53	65.53	78.00	
VNT 02	VID 03	994.00	883.00	481.66	1390	400	1.06	0.0020044	2.79	3.20	1059.53	1056.33	173.33	189.00	
VID 03	VNT 03	883.00	935.00	481.66	700	400	1.06	0.0020044	1.40	1.61	1056.33	1054.71	119.71	137.00	
VNT 03	VID 04	935.00	923.00	481.66	200	400	1.06	0.0020044	0.40	0.46	1054.71	1054.25	131.25	149.00	
VNT 03	VNT 04	923.00	1006.00	481.66	900	400	1.06	0.0020044	1.80	2.07	1054.25	1052.18	46.18	66.00	
VNT 04	PS3.1	1006.00	998.20	481.66	230	400	1.06	0.0020044	0.46	0.53	1052.18	1051.65	53.45	73.80	

Figure 26: The nominal pressure for stretch number 05 R Elmain - PS3.1

Stretch 06: PS3.2 – R Tafreg

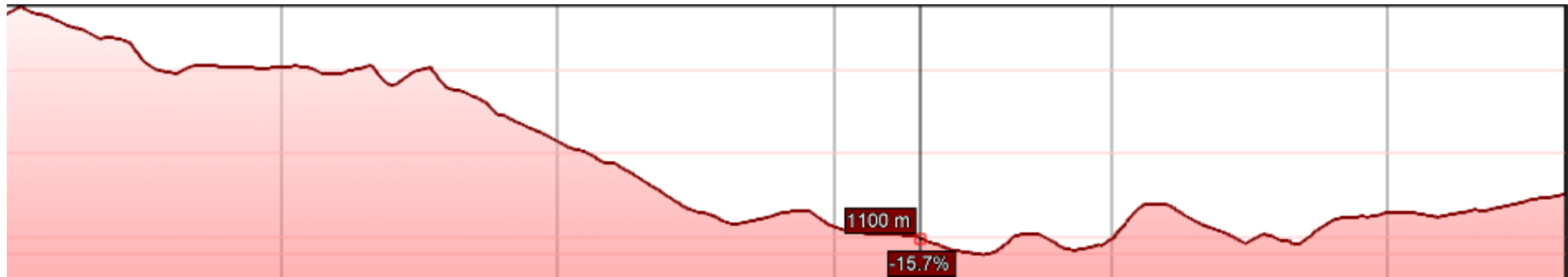


Figure 27: The longitudinal profile of the conduit PS3.2 - R Tafreg

Stretch		roughness of HDPE K=0,02mm			ΔHs=15% ΔHL										
		Cote (m)		flow (m3/h)	length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	Nominal Pressure
		upstream	downstream								upstream	downstream			
PS3.2	VID 01	1208.18	1214.00	47.17	173	130.8	0.98	0.0066240	1.15	1.32	1208.18	1206.86	-7.14	-5.82	NP16
VID 01	VNT 01	1214.00	1207.00	47.17	578	130.8	0.98	0.0066240	3.83	4.40	1206.86	1202.46	-4.54	1.18	
VNT 01	VID 02	1207.00	1090.00	47.17	1019	130.8	0.98	0.0066240	6.75	7.76	1202.46	1194.70	104.70	118.18	
VID 02	VNT 02	1090.00	1118.00	47.17	310	130.8	0.98	0.0066240	2.05	2.36	1194.70	1192.34	74.34	90.18	
VNT 02	R-TAFREG	1118.00	1157.48	47.17	810	130.8	0.98	0.0066240	5.37	6.17	1192.34	1186.17	28.69	50.70	

Figure 28: The nominal pressure for stretch number 06 PS3.2 – R Tafreg

Stretch 07: R Djaafra -Bif B



Figure 29: The longitudinal profile of the conduit R Djaafra - Bif B

Stretch		Cote (m)		flow (m3/h)	roughness of Steel K=0,1mm				ΔHs=15% ΔHL						
		upstream	downstream		length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	Nominal Pressure
R-DJAAFR	VID 01	1383.21	1313.00	263.04	736	200	2.33	0.0201993	14.87	17.10	1383.21	1366.11	53.11	70.21	NP20
VID 01	VNT 01	1313.00	1323.00	263.04	86	200	2.33	0.0201993	1.74	2.00	1366.11	1364.12	41.12	60.21	
VNT 01	VID 02	1323.00	1302.00	263.04	158	200	2.33	0.0201993	3.19	3.67	1364.12	1360.45	58.45	81.21	
VID 02	VNT 02	1302.00	1316.00	263.04	240	200	2.33	0.0201993	4.85	5.57	1360.45	1354.87	38.87	67.21	
VNT 02	VID 03	1316.00	1239.00	263.04	164	200	2.33	0.0201993	3.31	3.81	1354.87	1351.06	112.06	144.21	
VID 03	VNT 03	1239.00	1309.00	263.04	1556	200	2.33	0.0201993	31.43	36.14	1351.06	1314.92	5.92	74.21	
VNT 03	VNT 04	1309.00	1340.00	263.04	850	200	2.33	0.0201993	17.17	19.74	1314.92	1295.17	-44.83	43.21	
VNT 04	Bif-B	1340.00	1300.00	263.04	324	200	2.33	0.0201993	6.54	7.53	1295.17	1287.65	-12.35	83.21	

Figure 30: The nominal pressure for stretch number 07 R Djaafra -Bif B

Stretch 08: Bif B-R Colla



Figure 31: The longitudinal profile of the conduit Bif B - R Colla

Stretch		roughness of Steel K=0,1mm			ΔHs=15% AHL										
		Cote (m)		flow (m3/h)	length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	Nominal Pressure
		upstream	downstream								upstream	downstream			
Bif-B	VID 01	1300.00	1297.00	128.39	77.6	150	2.02	0.0220450	1.71	1.97	1300.00	1298.03	1.03	3.00	NP10
VID 01	VNT 01	1297.00	1303.00	128.39	112.4	150	2.02	0.0220450	2.48	2.85	1298.03	1295.18	-7.82	-3.00	
VNT 01	VID 02	1303.00	1257.00	128.39	636	150	2.02	0.0220450	14.02	16.12	1295.18	1279.06	22.06	43.00	
VID 02	VNT 02	1257.00	1263.00	128.39	58	150	2.02	0.0220450	1.28	1.47	1279.06	1277.59	14.59	37.00	
VNT 02	VID 03	1263.00	1226.00	128.39	274	150	2.02	0.0220450	6.04	6.95	1277.59	1270.64	44.64	74.00	
VID 03	R-COLLA	1226.00	1256.25	128.39	179	150	2.02	0.0220450	3.95	4.54	1270.64	1266.10	9.85	43.75	

Figure 32: The nominal pressure for stretch number 08 Bif B-R Colla

Stretch 09: Bif B - R Theniet enaser

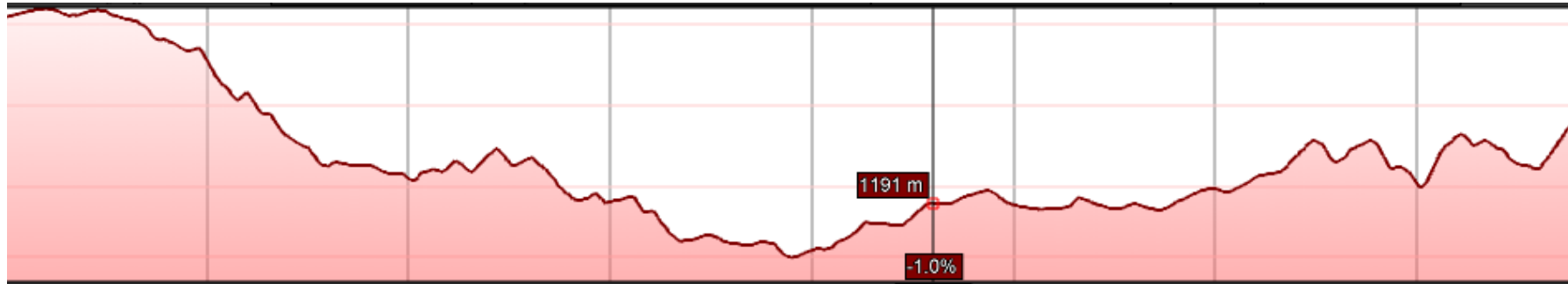


Figure 33: The longitudinal profile of the conduit Bif B - R Theniet enaser

Stretch		Cote (m)		flow (m3/h)	roughness of Steel K=0,1mm					ΔHs=15% ΔHL					Nominal Pressure
		upstream	downstream		length (m)	Inner diameter (mm)	Velocity (m/s)	J (m/m)	L Head loss (m)	Total head loss (m)	Hydraulic head (m)		Maximum operating pressure (m)	Static pressure (m)	
Bif-B	VID 01	1300.00	1204.00	134.65	1520	200	1.19	0.0058102	8.83	10.16	1300.00	1289.84	85.84	96.00	NP16
VID 01	VNT 01	1204.00	1224.00	134.65	310	200	1.19	0.0058102	1.80	2.07	1289.84	1287.77	63.77	76.00	
VNT 01	VID 02	1224.00	1157.00	134.65	1100	200	1.19	0.0058102	6.39	7.35	1287.77	1280.42	123.42	143.00	
VID 02	VNT 02	1157.00	1198.00	134.65	720	200	1.19	0.0058102	4.18	4.81	1280.42	1275.61	77.61	102.00	
VNT 02	VID 03	1198.00	1187.00	134.65	650	200	1.19	0.0058102	3.78	4.34	1275.61	1271.27	84.27	113.00	
VID 03	VNT 03	1187.00	1229.00	134.65	570	200	1.19	0.0058102	3.31	3.81	1271.27	1267.46	38.46	71.00	
VNT 03	VID 04	1229.00	1216.00	134.65	80	200	1.19	0.0058102	0.46	0.53	1267.46	1266.93	50.93	84.00	
VID 04	VNT 04	1216.00	1228.00	134.65	110	200	1.19	0.0058102	0.64	0.73	1266.93	1266.19	38.19	72.00	
VNT 04	VID 05	1228.00	1201.00	134.65	200	200	1.19	0.0058102	1.16	1.34	1266.19	1264.85	63.85	99.00	
VID 05	VNT 05	1201.00	1232.00	134.65	160	200	1.19	0.0058102	0.93	1.07	1264.85	1263.79	31.79	68.00	
VNT 05	VID 06	1232.00	1212.00	134.65	280	200	1.19	0.0058102	1.63	1.87	1263.79	1261.91	49.91	88.00	
VID 06	R-THENIET.ENASER	1212.00	1244.75	134.65	505	200	1.19	0.0058102	2.93	3.37	1261.91	1258.54	13.79	55.25	

Figure 34: The nominal pressure for stretch number 09 Bif B - R Theniet enaser

e-XC350-690/8000W/W45ADS4AG

Technical data	Company name	
	Contact	
	Phone number	
	e-mail address	

Operating data			
1	Pumpe type	Single pumps as parallel circuit	Fluid
2	No. of pumps	2	Operating temperature t A °C
3	Nominal flow	m ³ /h 2500	pH-value at t A
4	Nominal head	m 138.4	Density at t A kg/m ³
5	Static head	m 134.6	Kin. viscosity at t A mm ² /s
6	Inlet pressure	kPa 0	Vapor pressure at t A kPa
7	Environmental temperature	°C 20	Content of solid% Solid size mm
8	Available system NPSH	m 0	Altitude m

Pump data			
9	Design	Double Suction Split Case Pumps	Execution
10	Operating speed	rpm 1492	Clockwise Rotation - viewed from motor end [STD]
11	Number of stages	1	Max. mm
12	Suction nozzle	DNs 400 / EN1092-2 / PN40	designed mm
13	Discharge nozzle	DNd 350 / EN1092-2 / PN40	Min. mm
14	Connection	Type A - Flat Face	Nominal m ³ /h
15	Max. casing pressure	kPa 3100.1	Max- m ³ /h
16	Max. working pressure	kPa 1398.9	Min- m ³ /h
17	Impeller type	Radial impeller	Nominal m
18	Head H(Q=0)	m 140	at Qmax m
19	Max. shaft power	kW 1815.6	at Qmin m
20	Pump weight	kg 2,676.0	Shaft power kW
21	Total weight	kg 7,850.5	Efficiency %
			NPSH 3% m

Materials			
22		Pump	Shaft Seal
23	Casings	[D] - EN-GJS-500-7 / QT500-7 / ASTM A536, 80-55-06	Burgmann Rubber below seal
24	Impeller	[S] - 304 SS - 1.4308 / ZG0Cr18Ni9 / ASTM - CF8	MG12 - Seal on sleeve (ID 105/130 mm)
25	Shaft Construction	Dry(sleeves) [STD]	Mechanical seal diameter
26	Shaft	1.7035 / 40Cr / AISI - 5140	Rotating ring
27	Shaft Sleeves	304 SS - 1.4301 / 0Cr18Ni9 / AISI - 304	Stationary ring
28	Shaft Sleeve Nuts	304 SS - 1.4301 / 0Cr18Ni9 / AISI - 304	Elastomers
29	Casing Wear Ring	Bronze - CuSn8Zn4 / ASTM - C90300	Springs
30	Impeller Wear Ring	[S] - 304 SS - 1.4308 / ZG0Cr18Ni9 / ASTM - CF8	Other metal parts
31	Lantern Ring	Cast Iron	
32	Seal flush lines	304 SS - 1.4301 / 0Cr18Ni9 / AISI - 304	
33			
34			
35			
36			
37			
38			
39			
40			
41			

Motor data				Coupling	
42	Manufacturer	WEG		Manufacturer	Flender
43	Specific design	3ph Surface Motor - Low voltage		Series	N-EUPEX - Type A
44	Type	W50 450 L/K B3 800 kW		Type	A 315/4-105/130
45	Rated power	800 kW	Item no.	Frame size	315
46	Nominal speed	1492 rpm	Service factor	Spacer length	mm 4
47	Frame size	450 L/K	Electric voltage	Weight	kg 134.5
48	Weight	kg 4,355.0	Shaft diameter	Coupling protection	kg CG-06 35.001 kg

Base plate				Remarks	
49	Name	FRAME XC35-1172-450LK			
50	Weight	kg	650.0		

Project	Project ID	Created by	Created on	Last update
	Xylect-20156765	roumaissa bouras	06-11-24	06-11-24

e-XC350-690/8000WW45ADS4AG

Performance curve

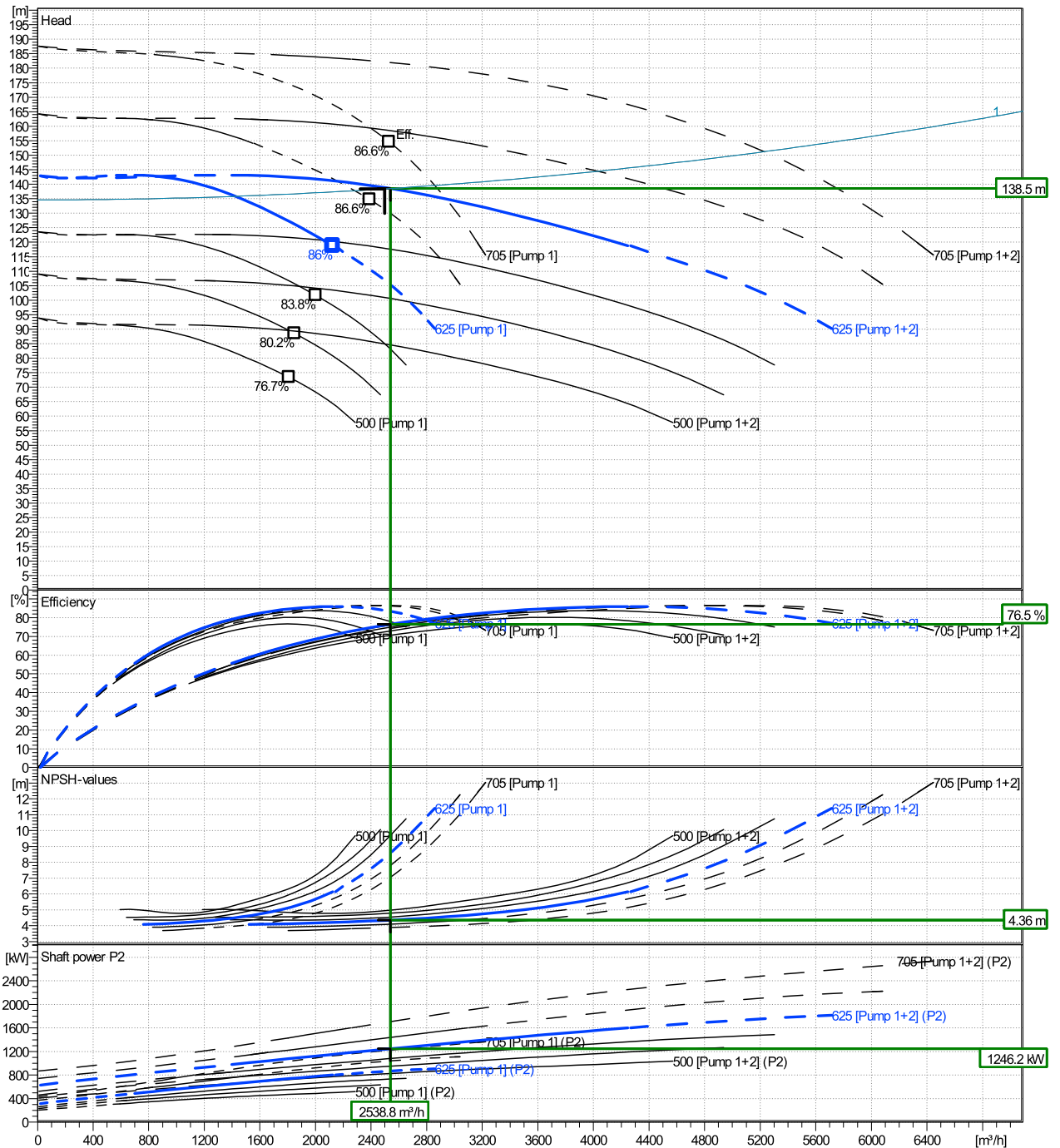
Company name
Contact
Phone number
e-mail address

	Ø inch	Pump capacity			Pump head		Shaft power P2			Frequency	Hz	
		Operating range Min. m³/h	Max. m³/h	η Max. m³/h	H(Q=0) m	η Max. m	P2(Q=0) kW	Max. kW	η Max. kW	Operating speed rpm	1492	
actual	625	761	2120	2120	143	119	1820	800		Nominal flow	m³/h	2500
Min.	500	/	/	1810	93.8	73.4	/	472		Nominal head	m	138.4
Max.	705	/	/	2530	188	155	/	1220		Inlet pressure	kPa	0
										Static head	m	134.6

Power datas referred to:

hydr. Performance acceptance acc. To EN ISO 9906 Class Grade 2B

Water, pure [100%] ; 20°C; 998kg/m³; 1mm²/s



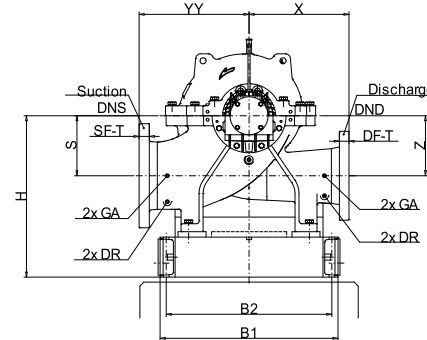
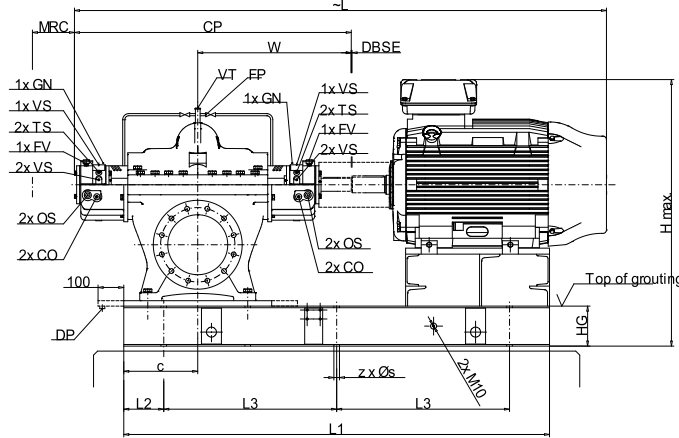
e-XC350-690/8000W/W45ADS4AG

Dimensions

Company name
Contact
Phone number
e-mail address

Complete Unit with Baseplate
Clockwise Rotation - viewed from motor end [STD]

3ph Surface Motor - Low voltage
W50 450 L/K B3 800 kW



- DR ... Drain Rp1/2
- FP ... Flush Rp3/8
- FV ... Fill/Vent Cup Rp3/8 standard
- GA ... Gauge Connection Rp1/2
- GN ... Grease Nipple M10x1 standard
- TS ... Temperature Sensor M10x1
- VT ... VentRp1/2
- VS ..

Rotation: CW View from Mtor to Pump

MRC Minimum removal clearance for bearing and seal maintenance
DBSE ... Distance between shaft ends

Dimensions [mm]

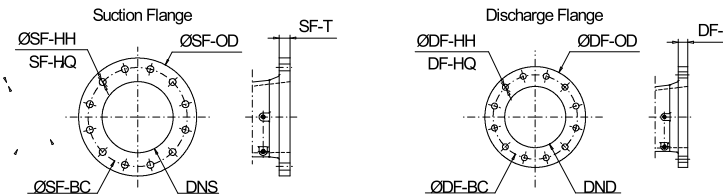
B1	1410
B2	1320
c	460
CP	1745
DBSE	4
H Max.	2087
H	1172
HG	300
L	3853
L1	3300
L2	300
L3	900
MRC	115
s	29
S	480
W	983
X	750
YY	900
Z	480
z	8x

Weight (+/- 5%) 7,850 kg

Connections [mm]

Suction nozzle		Discharge nozzle	
DNs 400		DNd 350	
PN40		PN40	
EN1092-2		EN1092-2	
Type A - Flat Face			
SF-BC	585	DF-BC	510
SF-HH	41	DF-HH	37
SF-HQ	16	DF-HQ	16
SF-OD	660	DF-OD	580
SF-T	57,2	DF-T	54

Flange drillings (SF-HH, SF-HQ, DF-HH, DF-HQ) according standard.
Outer diameter (SF-OD, DF-OD) according standard.
Flange thickness (SF-T, DF-T) varies from Standard.



Dimensions and weight without obligation

Project	Project ID Xylect-20156765	Created by roumaissa bouras	Created on 06-11-24	Last update 06-11-24
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IXPC125-100-315D750L25BDN4S1G

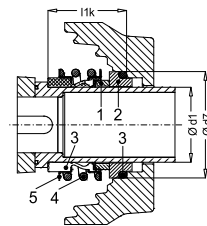
Technical data

Company name
Contact
Phone number
e-mail address

Operating data			
1	Pumpe type	Single pumps as serial circuit	Fluid Water, pure
2	No. of pumps	2	Operating temperature t A °C 20
3	Nominal flow	m ³ /h 116.12	Max / Min Operating Temperature °C 120 / -40
4	Nominal head	m 209.18	pH-value at t A 7
5	Static head	m 171	Density at t A kg/m ³ 998
6	Inlet pressure	kPa 0	Kin. viscosity at t A mm ² /s 1
7	Environmental temperature	°C 20	Vapor pressure at t A kPa 100
8	Available system NPSH	m 0	Altitude 0

Pump data			
9	Lubrication	Grease Lubrication [Std] (Size 42)	
10	Execution	Spacer coupling	
11	Design	Horizontal	
12	Operating speed	2985 rpm	
13	Suction nozzle	EN1092-2 DNs 125 PN10/16	Impeller Ø
14	Discharge nozzle	EN1092-2 DNd 100 PN10/16	
15	Max. casing pressure	kPa 1600	Flow
16	Max. working pressure	kPa 1028.4	
17	Impeller type	Radial impeller	
18	Head H(Q=0)	m 110	Head
19	Max. shaft power	kW 219.9	
20	Pump weight	kg 165	Shaft power kW 115.2
21	Total weight	kg 946.0	Efficiency % 57.6
			NPSH 3% m 3.4

Materials			
22		Pump	Shaft Seal
23	Impeller (1)	Austenitic Stainless Steel, 1.4408, ASTM CF8M	[S1] Single Mechanical Seal - Unbalanced
24	Discharge Casing (2)	Ductile Iron, EN-GJS-400-15, ASTM 65-45-12	eMG12 - Ø53mm BQ7EGG
25	Casing Cover (3)	Ductile Iron, EN-GJS-400-15, ASTM 65-45-12	Mechanical seal diameter 53 mm
26	Seal Cover (4)	not applicable for selected seal configuration	1. Rotating ring Carbon graphite resin impregnated
27	Wear Ring (5)	Duplex Stainless Steel, 1.4462/1.4517	2. Stationary ring SiC, silicon carbide, sintered press.less
28	Impeller Nut (6)	Duplex Stainless Steel, 1.4517, ASTM CD4MCuN	3. Secondary seal Ethylene propylene rubber (EPDM)
29	Quench Labyrinth Ring (7)	not applicable for selected seal configuration	4. Springs CrNiMo - Steel
30	Shaft Sleeve (8)	Duplex Stainless Steel, 1.4462, ASTM F51	5. Others CrNiMo - Steel
31	Shaft (9)	Stainless Steel, 1.4057, ASTM 431	Supplier Code BQ7EGG
32	Bearing Bracket (11)	Cast Iron, EN-GJL-250, ASTM Class 35	
33	Bearing Cover (12)	Cast Iron, EN-GJL-250, ASTM Class 35	
34	Pump Support Foot (14)	Carbon Steel, 1.0038, ASTM Grade C, D	
35	O-Ring (16)	PTFE	
36	O-Ring Casing Cover (17)	Ethylene propylene rubber (EPDM)	
37	Plug (19)	Austenitic Stainless Steel	
38	Key (21)	Stainless Steel, 1.4571, ASTM - 316Ti	
39	Screw & Nut (22)	Austenitic Stainless Steel	
40			
41	Shaft Sealing	Radial Shaft Seal Ring - FKM	



Motor data				Coupling			
42	Manufacturer	Lowara		Manufacturer	Flender		
43	Specific design	IE4 3ph Surface Motor - Super Premium Efficiency		Series	Spacer Coupling - N-EUPEX - Type H		
44	Type	3MGS 280 S B3 75 kW E4		Spacer length	140 mm	Suitable for EEx-design	NO
45	Rated power	75 kW	Rated current 129 A	Shaft diameter	Motor	65 mm	Frame size 160
46	Nominal speed	2985 rpm	Rated voltage 400 V		Pump	42 mm	Weight 15.2 kg
47	Frame size	280 S	Service factor 1	GR.COUPLGUARD ES42-230-37 A4			
48	Weight	kg 596.0	Degree of protection IP55	Coupling protection	Material	Edelstahl	
49	Length shaft end	140 mm	Design acc. standard IEC		Weight	4.6 kg	

Base plate		Remarks
50	Name	FRAME 42B250C-315C280
51	Weight	kg 165.2
52	Drip Pan	Without Drip Pan

IXPC125-100-315D750L25BDN4S1G

Performance curve

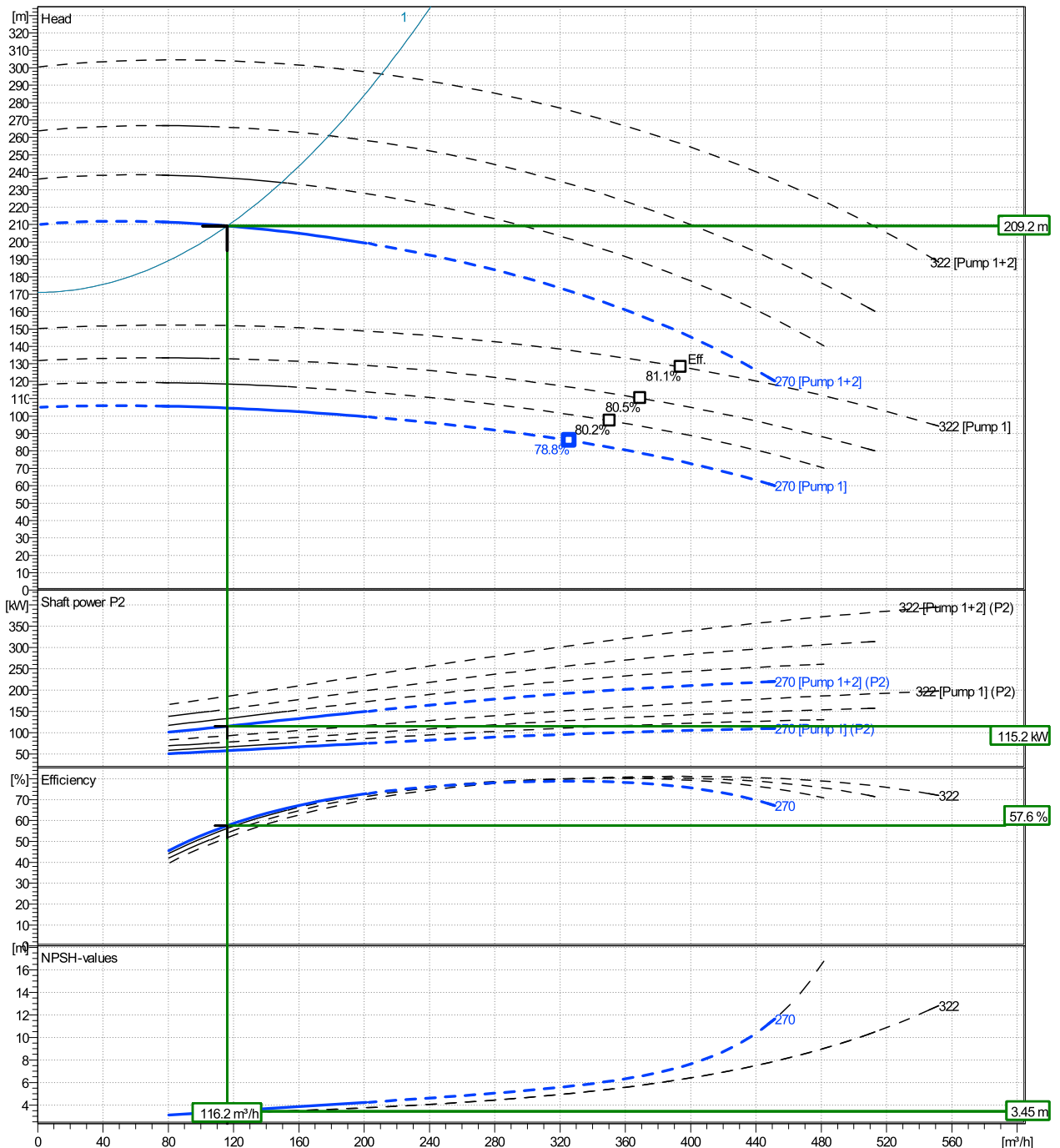
Company name
Contact
Phone number
e-mail address

	Ø mm	Pump capacity			Pump head		Shaft power P2			Frequency	Hz	
		Operating range Min. m³/h	Max. m³/h	η Max. m³/h	H(Q=0) m	η Max. m	P2(Q=0) kW	Max. kW	η Max. kW	Operating speed rpm	2985	
actual	270	80.3	201	326	105	86	220	96.3		Nominal flow	m³/h	116.1
Min.	270	/	/	326	105	86	/	96.3		Nominal head	m	209.2
Max.	322	/	/	394	150	128	/	169		Inlet pressure	kPa	0
										Static head	m	171

Power data referred to:
Water, pure [100%] ; 20°C; 998kg/m³; 1mm²/s

hydr. Performance acceptance acc. To EN ISO 9906 Class 2B

MEI: N.A - according to Ecodesign Directive 2009/125/EC and Regulation (EU) No.547/2012



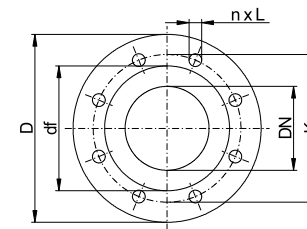
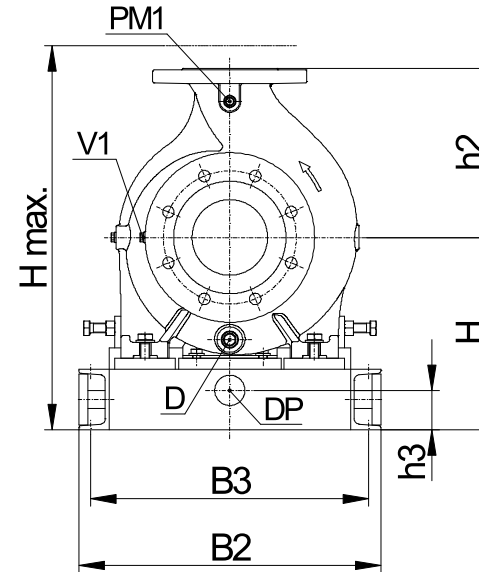
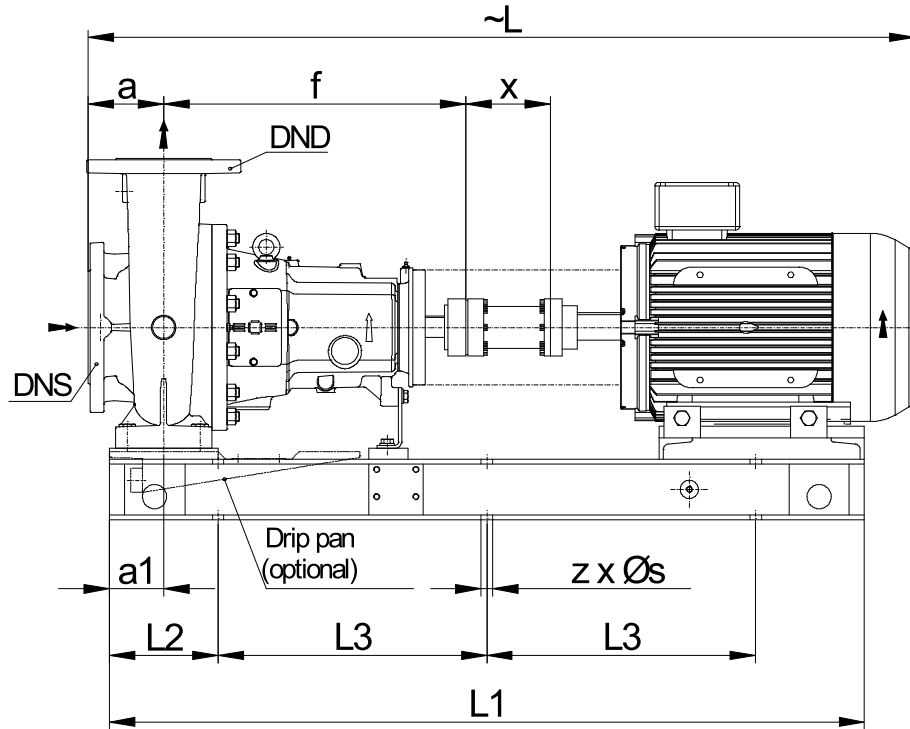
IXPC125-100-315D750L25BDN4S1G

Dimensions

Company name
Contact
Phone number
e-mail address

Frame mounted
Spacer coupling
3MGS 280 S B3 75 kW E4

Electrical and dimensional data refer to IE4 motor



Dimensions [mm]			
a	140	s	26
a1	90	Volumen	1.1795
B2	830	x	140
B3	780	z	6
DNd	100		
DNs	125		
f	530		
Guard	390		
H max.	856		
H	443		
h2	315		
h3	140		
L	1766.5		
L1	1600		
L2	200		
L3	600		

Weight (+/- 5%)	
Pump	165 kg
Coupling	15.2
Base plate	165
Motor	596
Total weight	946 kg

Connections			
Suction nozzle		Discharge nozzle	
DNs 125		DNd 100	
PN10/16		PN10/16	
EN1092-2		EN1092-2	
C	26	C	24
D	280	D	255
df	186	df	156
K	210	K	180
n	8x	n	8x
ØL	19	ØL	19

Connections:
D: Drain G1/2"
DP: Drip pan connection G1" (optional)
V1: Pressure gauge connector G1/4" (suction side)
PM1: Pressure gauge connector G1/4" (discharge side)

Note:
Value D, C and df may vary from standard

Dimensions and weight without obligation

Project	Xylect-20156793	Created by	roumaissa bouras	Last update	6/11/2024
Block	IXPC125-100-315D750L25BDN4S1G	Created on	6/11/2024		

MPA100A/08A/BD750/L45VCCC4

Technical data

Company name
Contact
Phone number
e-mail address

Operating data			
1	Pumpe type	Single pumps as parallel circuit	Fluid Water, pure
2	No. of pumps	3	Operating temperature t A °C 20
3	Nominal flow	m³/h 116.1	Max / Min Operating Temperature °C 140 / -25
4	Nominal head	m 159.1	Max / Min Operating Temperature mech. Seal °C 120 / -10
5	Static head	m 150.7	pH-value at t A 7
6	Inlet pressure / Vapor pressure at t A kPa	0 / 100	Density at t A kg/m³ 998
7	Environmental temperature	°C 20	Kin. viscosity at t A mm²/s 1
8	Available system NPSH	m 0	Altitude 0

Pump data				
9	Design	Horizontal Multistage pump, axial DN, radial DNd, 1 Slide bearing DN, 1 Roller bearings DNd		
10	Execution	AO / DN - axial, DNd - above		
11	Lubrication	Grease Lubrication		
12	Operating speed	rpm 1490	Stages 8	
13	Flange connections	EN1092-2 / DN150, PN10/16 - DN100, PN25/40		
14	Max. casing pressure	kPa 4000	Impeller Ø Max. mm 242 designed mm 8x 242 mm Min. mm 176 Nominal m³/h 120.3 Max- m³/h 102.8 Min- m³/h 20.6	
15	Max. working pressure	kPa 1661.6		
16	Impeller type	Radial impeller		
17	Head H(Q=0)	m 170		
18	Max. shaft power	kW 102.8	Flow Nominal m 159.7 at Qmax m 71.4 at Qmin m 168.2	
19	Pump weight	kg 470		
20	Total weight	kg 1,386.7	Head Shaft power kW 76.9 Efficiency % 67.8 NPSH 3% m 1.1	

Materials			
22		Pump	Shaft Seal
23	Suction Impeller	Cast Iron, EN-GJL-200, ASTM Class 30	Single mechanical seal, with shaft sleeve (unbalanced)
24	Impeller	Cast Iron, EN-GJL-200, ASTM Class 30	eMG12 - Ø55mm BQ7EGG-WA
25	Diffuser	Cast Iron, EN-GJL-150, ASTM Class 25	Mechanical seal diameter 55 mm
26	Stage Casing	Cast Iron, EN-GJL-250, ASTM Class 35	1. Rotating ring Carbon graphite resin impregnated
27	Suction Casing	Cast Iron, EN-GJL-250, ASTM Class 35	2. Stationary ring SiC, silicon carbide, sintered press. less
28	Discharge Casing	Cast Iron, EN-GJL-250, ASTM Class 35	3. Secondary seal Ethylene propylene rubber (EPDM)
29	Seal Cover	Cast Iron, EN-GJL-250, ASTM Class 35	4. Springs CrNiMo - Steel
30	Bearing Bracket / Motor Adapter	Cast Iron, EN-GJL-250, ASTM Class 35	5. Others EPDM - WRAS
31	Pump Foot	Cast Iron, EN-GJL-250, ASTM Class 35	Gaskets of the pump Ethylene propylene rubber (EPDM)
32	Wear ring	without [STD]	
33	Drum	Stainless Steel, 1.4057, ASTM 431	
34	Drum Bush	Cast Iron, EN-GJL-250, ASTM Class 35	
35	Shaft	Stainless Steel, 1.4057, ASTM 431	
36	Shaft Sleeve	Stainless Steel, 1.4057, ASTM 431	
37	Spacer Sleeve	Stainless Steel, 1.4057, ASTM 431	
38	Shaft Nut	Stainless Steel, 1.4057, ASTM 431	
39	Impeller Nut	A4	
40			
41			
42			
43			

Motor data		Electrical and dimensional data refer to IE4 motor		Coupling	
44	Manufacturer	Lowara		Manufacturer	FLENDER
45	Specific design	IE4 3ph Surface Motor - Super Premium Efficiency		Series	Standard Coupling - N-EUPEX - Type B
46	Type	3MGS 280 S B3 75 kW E4		Shaft diameter Pump / Motor mm	45.0 / 75.0
47	Rated power	75 kW	Item no.	Frame size	180
48	Nominal speed	1490 rpm	Service factor	Spacer length mm	4
49	Frame size	280 S	Electric data	Weight kg	11.5
50	Weight kg	702.0	Shaft diameter	Coupling protection	e-MP_GR.COUPGUARD ES42-230-23 A4 3.4 kg
Base plate				Remarks	
51	Name	FRAME EMP100-HJ-280			
52	Weight	kg	199.8		

Project	Xylect-20156819	Created by	roumaissa bouras	Last update	6/11/2024
Block	MPA100A/08A/BD750/L45VCCC4	Created on	6/11/2024		

MPA100A/08A/BD750/L45VCCC4

Performance curve

Company name
Contact
Phone number
e-mail address

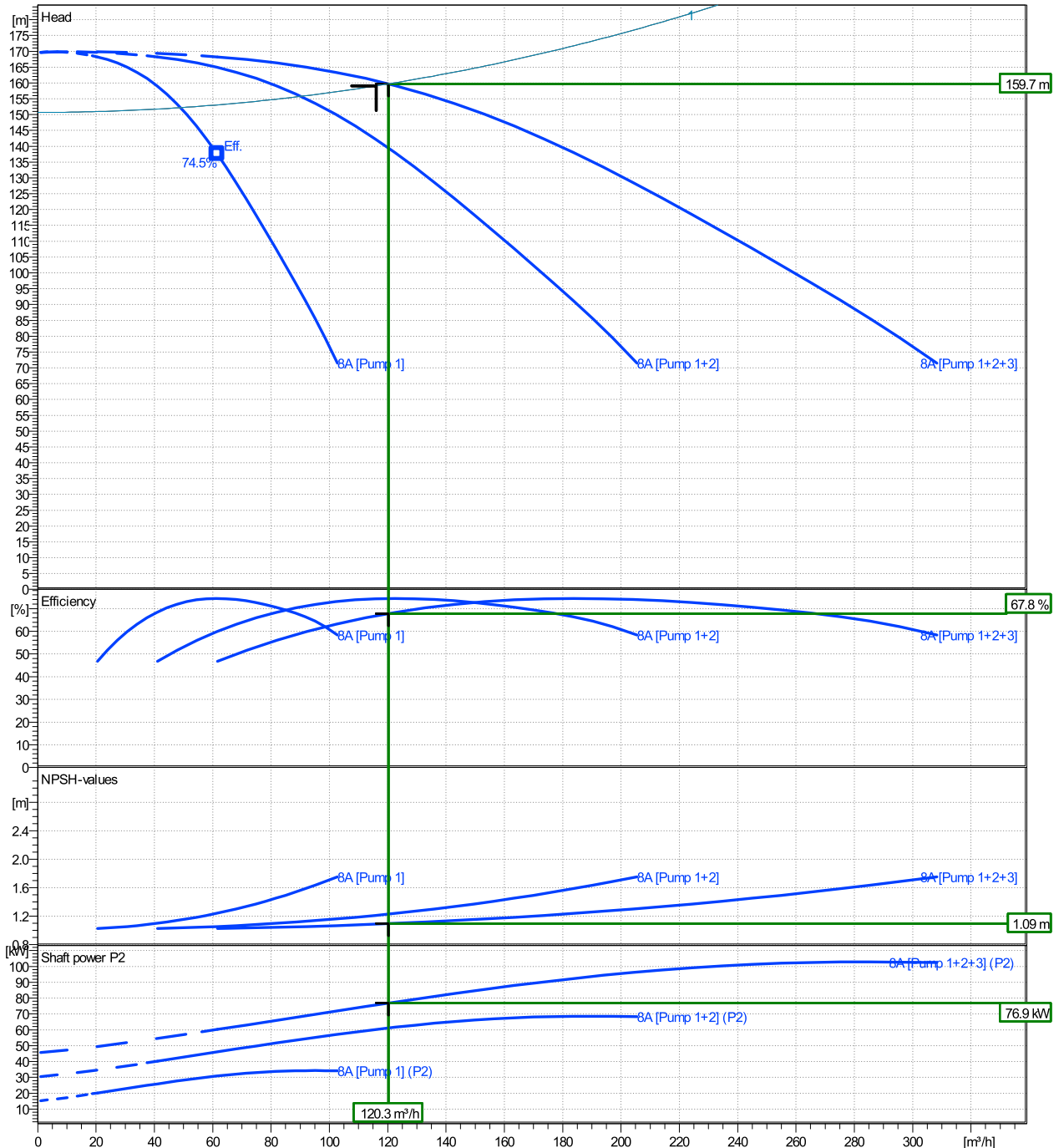
	Ø mm	Pump capacity Operating range			Pump head		Shaft power P2			Frequency		Hz	50
		Min. m³/h	Max. m³/h	η Max. m³/h	H(Q=0) m	η Max. m	P2(Q=0) kW	Max. kW	η Max. kW	Operating speed rpm	Nominal flow m³/h		
actual	242	20.6	103	61.5	170	138	103	30.9	30.9	Nominal head	m	159.1	
Min.	176	/	/	61.5	170	138	/	/	30.9	Inlet pressure	kPa	0	
Max.	242	/	/	61.5	170	138	/	/	30.9	Static head	m	150.7	

Power datas referred to:

hydr. Performance acceptance acc. To EN ISO 9906 Class Grade 2B

Water, pure [100%]; 20°C; 998kg/m³; 1mm²/s

MEI: N.A - according to Ecodesign Directive 2009/125/EC and Regulation (EU) No.547/2012



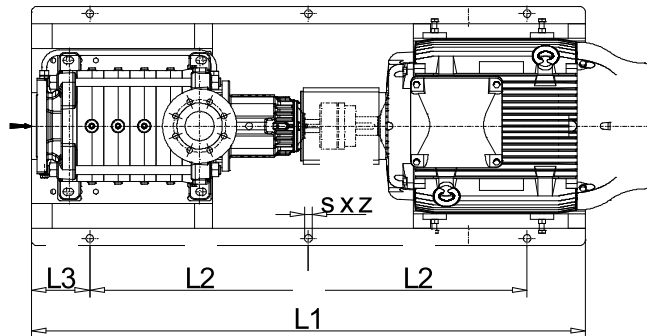
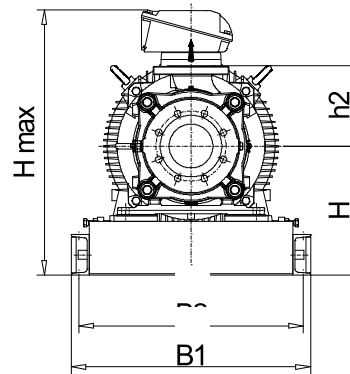
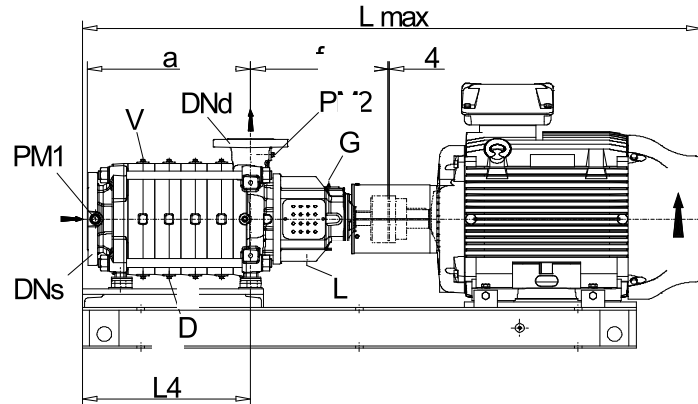
MPA100A/08A/BD750/L45VCCC4

Dimensions

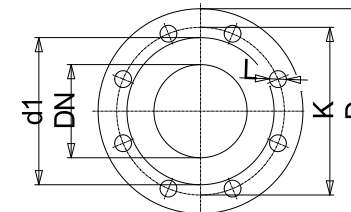
Company name
Contact
Phone number
e-mail address

Complete Unit with Baseplate
AO / DN_s - axial, DN_d - above
3MGS 280 S B3 75 kW E4

Electrical and dimensional data refer to IE4 motor



PM1...Pressure gauge connector
PM2...Pressure gauge connector
D...Drain
G...Grease nipple
L...Leakage
V...Venting



Note: Value D, C and d may vary from standard

Dimensions [mm]			
a	830	L4	937
B1	820	Lmax	2369.5
B2	770	PM1	G1/4
D	G1/4	PM2	G1/4
DN _d	100	s	26
DN _s	150	V	G1/4
f	472	Volumen	1.6632
G	M8	z	6
Guard	254		
H	443		
h2	275		
Hmax	856		
L	G1/2		
L1	2250		
L2	925		
L3	200		

Weight (+/- 5%)	
Pump	470
Coupling	11.5
Base plate	200
Motor	702
Total weight	1,387 kg

Connections			
Suction nozzle		Discharge nozzle	
DN _s 150		DN _d 100	
PN10/16		PN25/40	
EN1092-2		EN1092-2	
C	34	C	28
D	320	D	255
d1	211	d1	156
K	240	K	190
L	23	L	23
z	8	z	8

Dimensions and weight without obligation

Project	Xylect-20156819	Created by	roumaissa bouras	Last update	6/11/2024
Block	MPA100A/08A/BD750/L45VCCC4	Created on	6/11/2024		

MPA125A/02A/BD1600/L25VCCC4

Technical data

Company name
Contact
Phone number
e-mail address

Operating data			
1	Pumpe type	Single pumps as parallel circuit	Fluid Water, pure
2	No. of pumps	4	Operating temperature t A °C 20
3	Nominal flow	m³/h 612.8	Max / Min Operating Temperature °C 140 / -25
4	Nominal head	m 205.8	Max / Min Operating Temperature mech. Seal °C 120 / -10
5	Static head	m 199	pH-value at t A 7
6	Inlet pressure / Vapor pressure at t A kPa	0 / 100	Density at t A kg/m³ 998
7	Environmental temperature	°C 20	Kin. viscosity at t A mm²/s 1
8	Available system NPSH	m 0	Altitude 0

Pump data				
9	Design	Horizontal Multistage pump, axial DN's, radial DNd, 1 Slide bearing DN's, 1 Roller bearings DNd		
10	Execution	AO / DN's - axial, DNd - above		
11	Lubrication	Grease Lubrication		
12	Operating speed	rpm 2985	Stages 2	
13	Flange connections	EN1092-2 / DN's200, PN16 - DNd125, PN25/40		
14	Max. casing pressure	kPa 2500	Impeller Ø Max. mm 283 designed mm 2x 283 mm Min. mm 223 Nominal m³/h 612.8 Max- m³/h 394.6 Min- m³/h 121.4	
16	Max. working pressure	kPa 2228.3		
17	Impeller type	Radial impeller		
18	Head H(Q=0)	m 230		
19	Max. shaft power	kW 540	Flow Max- m³/h 394.6 Min- m³/h 121.4	
20	Pump weight	kg 430		
21	Total weight	kg 1,635.4	Head Nominal m 205.8 at Qmax m 113.6 at Qmin m 222.8	
			Shaft power kW 448.7	
			Efficiency % 76.4	
			NPSH 3% m 4.5	

Materials			
22		Pump	Shaft Seal
23	Suction Impeller	Cast Iron, EN-GJL-200, ASTM Class 30	Single mechanical seal, with shaft sleeve (unbalanced)
24	Impeller	Cast Iron, EN-GJL-200, ASTM Class 30	eMG12 - Ø65mm BQ7EGG-WA
25	Diffuser	Cast Iron, EN-GJL-150, ASTM Class 25	Mechanical seal diameter 65 mm
26	Stage Casing	Cast Iron, EN-GJL-250, ASTM Class 35	1. Rotating ring Carbon graphite resin impregnated
27	Suction Casing	Cast Iron, EN-GJL-250, ASTM Class 35	2. Stationary ring SiC, silicon carbide, sintered press. less
28	Discharge Casing	Cast Iron, EN-GJL-250, ASTM Class 35	3. Secondary seal Ethylene propylene rubber (EPDM)
29	Seal Cover	Cast Iron, EN-GJL-250, ASTM Class 35	4. Springs CrNiMo - Steel
30	Bearing Bracket / Motor Adapter	Cast Iron, EN-GJL-250, ASTM Class 35	5. Others EPDM - WRAS
31	Pump Foot	Carbon Steel, 1.0038, ASTM Grade C, D	Gaskets of the pump Ethylene propylene rubber (EPDM)
32	Wear ring	without [STD]	
33	Drum	Stainless Steel, 1.4057, ASTM 431	
34	Drum Bush	Cast Iron, EN-GJL-250, ASTM Class 35	
35	Shaft	Stainless Steel, 1.4057, ASTM 431	
36	Shaft Sleeve	Stainless Steel, 1.4057, ASTM 431	
37	Spacer Sleeve	Stainless Steel, 1.4057, ASTM 431	
38	Shaft Nut	Stainless Steel, 1.4057, ASTM 431	
39	Impeller Nut	A4	
40			
41			
42			
43			

Motor data		Electrical and dimensional data refer to IE4 motor		Coupling	
44	Manufacturer	Lowara		Manufacturer	FLENDER
45	Specific design	IE4 3ph Surface Motor - Super Premium Efficiency		Series	Standard Coupling - N-EUPEX - Type B
46	Type	3MGS 315 M B3 160 kW E4		Shaft diameter Pump / Motor mm	52.0 / 65.0
47	Rated power	160 kW	Item no.	Frame size	160
48	Nominal speed	2985 rpm	Service factor 1	Spacer length mm	4
49	Frame size	315 M	Electric data 400 V 262 A	Weight kg	7.8
50	Weight kg	955.0	Shaft diameter 65.0 mm	Coupling protection	e-MP_GR.COUPPLGUARD ES42-230-23 A4 3.4 kg
Base plate				Remarks	
51	Name	FRAME EMP125-B-312			
52	Weight kg	239.2			

Project	Xylect-20156843	Created by	roumaissa bouras	Last update	6/11/2024
Block	MPA125A/02A/BD1600/L25VCCC4	Created on	6/11/2024		

MPA125A/02A/BD1600/L25VCCC4

Performance curve

Company name
Contact
Phone number
e-mail address

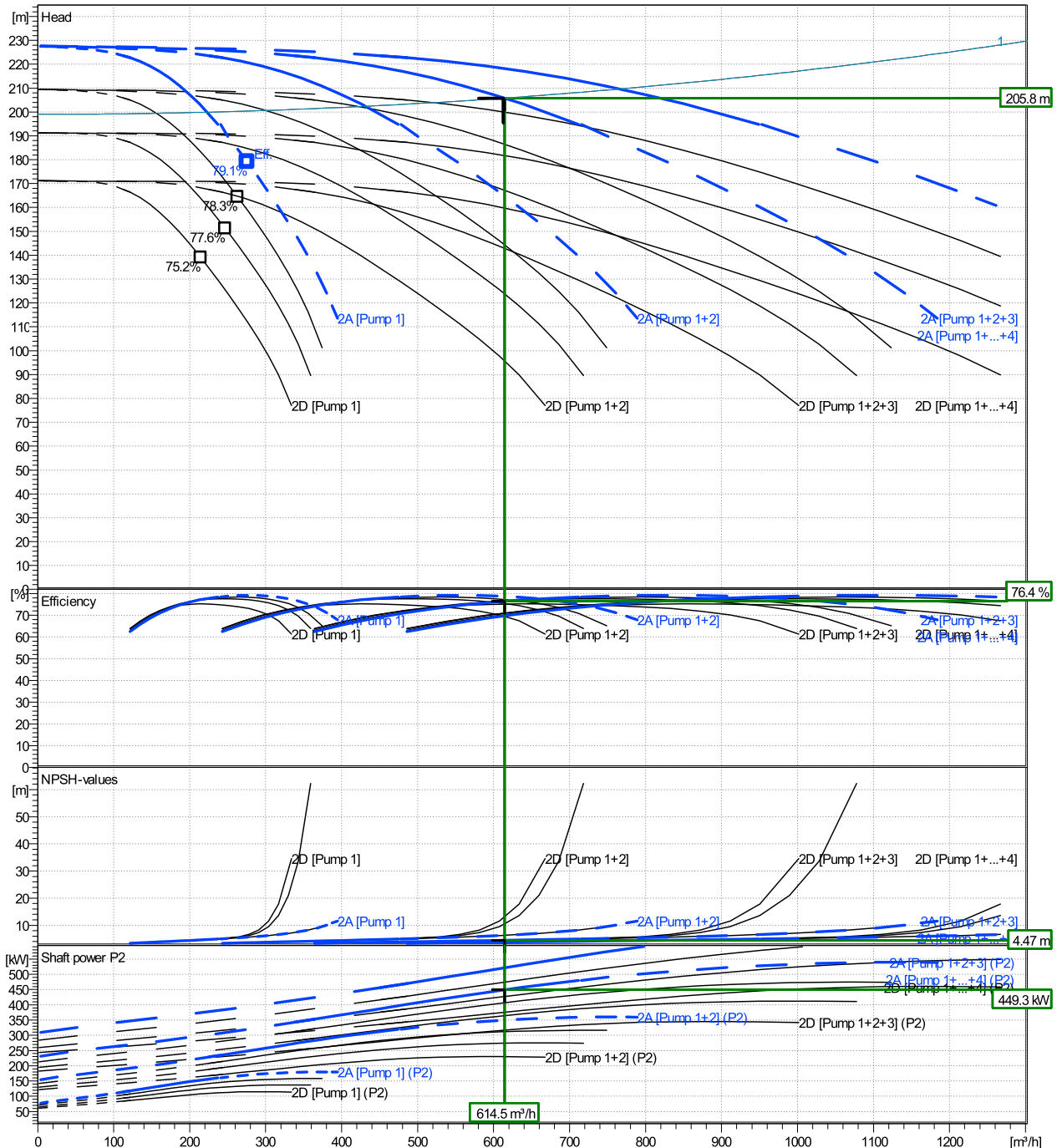
	Ø mm	Pump capacity			Pump head		Shaft power P2			Frequency	Hz	
		Operating range Min. m³/h	Max. m³/h	η Max. m³/h	H(Q=0) m	η Max. m	P2(Q=0) kW	Max. kW	η Max. kW	Operating speed rpm	2985	
actual	283	138	237	276	228	179	540	169	108	Nominal flow	m³/h	612.8
Min.	223	/	/	215	171	139	/	/	108	Nominal head	m	205.8
Max.	283	/	/	276	228	179	/	/	169	Inlet pressure	kPa	0
										Static head	m	199

Power datas referred to:

hydr. Performance acceptance acc. To EN ISO 9906 Class Grade B2B

Water, pure [100%]; 20°C; 998kg/m³; 1mm²/s

MEI: N.A - according to Ecodesign Directive 2009/125/EC and Regulation (EU) No.547/2012



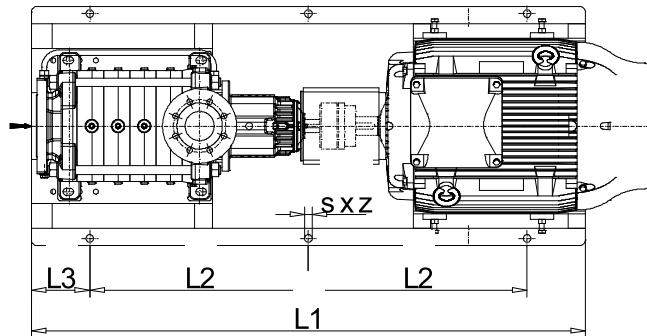
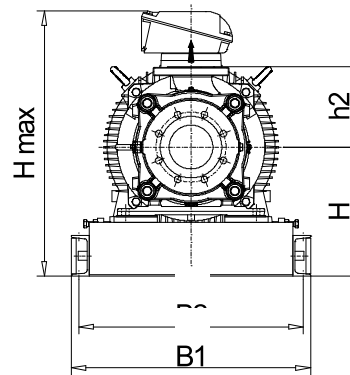
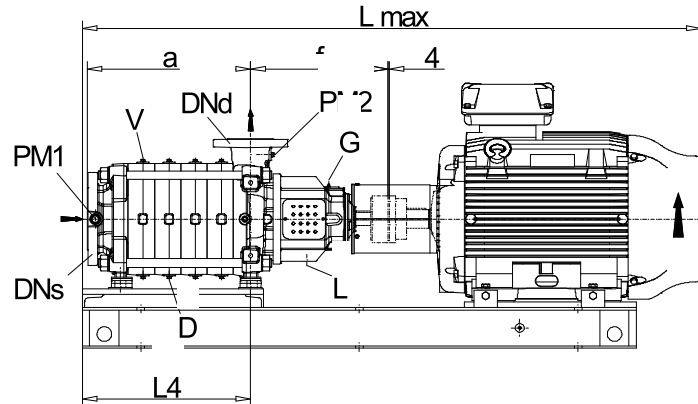
MPA125A/02A/BD1600/L25VCCC4

Dimensions

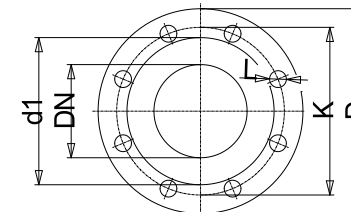
Company name
Contact
Phone number
e-mail address

Complete Unit with Baseplate
AO / DN_s - axial, DN_d - above
3MGS 315 M B3 160 kW E4

Electrical and dimensional data refer to IE4 motor



PM1...Pressure gauge connector
PM2...Pressure gauge connector
D...Drain
G...Grease nipple
L...Leakage
V...Venting



Note: Value D, C and d may vary from standard

Dimensions [mm]			
a	388	L4	390
B1	860	Lmax	2008
B2	810	PM1	G1/4
D	G1/4	PM2	G1/4
DN _d	125	s	26
DN _s	200	V	G1/4
f	488	Volumen	1.7839
G	M8	z	6
Guard	254		
H	508		
h2	325		
Hmax	1033		
L	G1/2		
L1	1950		
L2	775		
L3	200		

Weight (+/- 5%)	
Pump	430
Coupling	7.8
Base plate	239
Motor	955
Total weight	1,635 kg

Connections			
Suction nozzle		Discharge nozzle	
DN _s 200		DN _d 125	
PN16		PN25/40	
EN1092-2		EN1092-2	
C	40	C	30
D	380	D	280
d1	266	d1	184
K	295	K	220
L	23	L	28
z	12	z	8

Dimensions and weight without obligation

Project	Xylect-20156843	Created by	roumaissa bouras	Last update	6/11/2024
Block	MPA125A/02A/BD1600/L25VCCC4	Created on	6/11/2024		

MPA150A/02C/BD3150/L25VDNC4

Technical data

Company name
Contact
Phone number
e-mail address

Operating data							
1	Pumpe type	Single pumps as parallel circuit		Fluid	Water, pure		
2	No. of pumps	3		Operating temperature at t A	°C	20	
3	Nominal flow	m³/h	612.8	Max / Min Operating Temperature	°C	140 / -25	
4	Nominal head	m	235.85	Max / Min Operating Temperature mech. Seal	°C	120 / -10	
5	Static head	m	233	pH-value at t A	7		
6	Inlet pressure / Vapor pressure at t A	kPa	0 / 100	Density at t A	kg/m³	998	
7	Environmental temperature	°C	20	Kin. viscosity at t A	mm²/s	1	
8	Available system NPSH	m	0	Altitude	0		

Pump data									
9	Design	Horizontal Multistage pump, axial DNs, radial DNd, 1 Slide bearing DNs, 1 Roller bearings DNd							
10	Execution	AO / DNs - axial, DNd - above							
11	Lubrication	Grease Lubrication		Impeller Ø	Max.	mm	334		
12	Operating speed	rpm	2985		Stages	2	designed	mm	2x 304 mm
13	Flange connections	EN1092-2 - DNs250, PN16 - DNd150, PN25/40							
14	EN1092-2 - DNs250, PN16 - DNd150, PN25/40			Flow	Min.	mm	265		
15	Max. casing pressure	kPa	4000		Nominal	m³/h	612.8		
16	Max. working pressure	kPa	2527.7		Max-	m³/h	602.1		
17	Impeller type	Radial impeller		Head	Min-	m³/h	253		
18	Head H(Q=0)	m	260		Nominal	m	235.9		
19	Max. shaft power	kW	617.8		at Qmax	m	133.6		
20	Pump weight	kg	701	at Qmin	m	245.6			
21	Total weight	kg	2,647.0	Shaft power	kW	538.2			
				Efficiency	%	72.6			
				NPSH 3%	m	6.7			

Materials			
22		Pump	Shaft Seal
23	Suction Impeller	Austenitic Stainless Steel, 1.4408, ASTM CF8M	Single mechanical seal, with shaft sleeve (balanced)
24	Impeller	Austenitic Stainless Steel, 1.4408, ASTM CF8M	H75N - Ø75mm
25	Diffuser	Cast Iron, EN-GJL-150, ASTM Class 25	Mechanical seal diameter
26	Stage Casing	Ductile Iron, EN-GJS-400-15, ASTM 65-45-12	75 mm
27	Suction Casing	Ductile Iron, EN-GJS-400-15, ASTM 65-45-12	1. Rotating ring
28	Discharge Casing	Ductile Iron, EN-GJS-400-15, ASTM 65-45-12	2. Stationary ring
29	Seal Cover	Ductile Iron, EN-GJS-400-15, ASTM 65-45-12	3. Secondary seal
30	Bearing Bracket / Motor Adapter	Cast Iron, EN-GJL-250, ASTM Class 35	4. Springs
31	Pump Foot	Carbon Steel, 1.0038, ASTM Grade C, D	5. Others
32	Wear ring	without [STD]	Gaskets of the pump
33	Drum	Stainless Steel, 1.4057, ASTM 431	Ethylene propylene rubber (EPDM)
34	Drum Bush	Cast Iron, EN-GJL-250, ASTM Class 35	
35	Shaft	Stainless Steel, 1.4057, ASTM 431	
36	Shaft Sleeve	Stainless Steel, 1.4057, ASTM 431	
37	Spacer Sleeve	Stainless Steel, 1.4057, ASTM 431	
38	Shaft Nut	Stainless Steel, 1.4057, ASTM 431	
39	Impeller Nut	A4	
40			
41			
42			
43			

Motor data				Coupling			
44	Manufacturer	Lowara		Manufacturer	FLENDER		
45	Specific design	IE3 3ph Surface Motor - Premium Efficiency		Series	Spacer Coupling - N-EUPEX - Type H		
46	Type	3MGS 355 M B3 315 kW		Shaft diameter	Pump / Motor	mm	60.0 / 75.0
47	Rated power	315 kW	Item no.	Frame size	200		
48	Nominal speed	2985 rpm	Service factor	Spacer length	mm	250	
49	Frame size	355 M	Electric data	Weight	kg	32.1	
50	Weight	kg	1,547.0	Shaft diameter	75.0	mm	Coupling protection
							e-MP_GR.COUPGUARD ES60-310-54 A4 6.3 kg
Base plate				Remarks			
51	Name	FRAME EMP150-B-S-355					
52	Weight	kg	360.6				

Project	Xylect-20156882	Created by	roumaissa bouras	Last update	6/11/2024
Block	MPA150A/02C/BD3150/L25VDNC4	Created on	6/11/2024		

MPA150A/02C/BD3150/L25VDNC4

Performance curve

Company name
Contact
Phone number
e-mail address

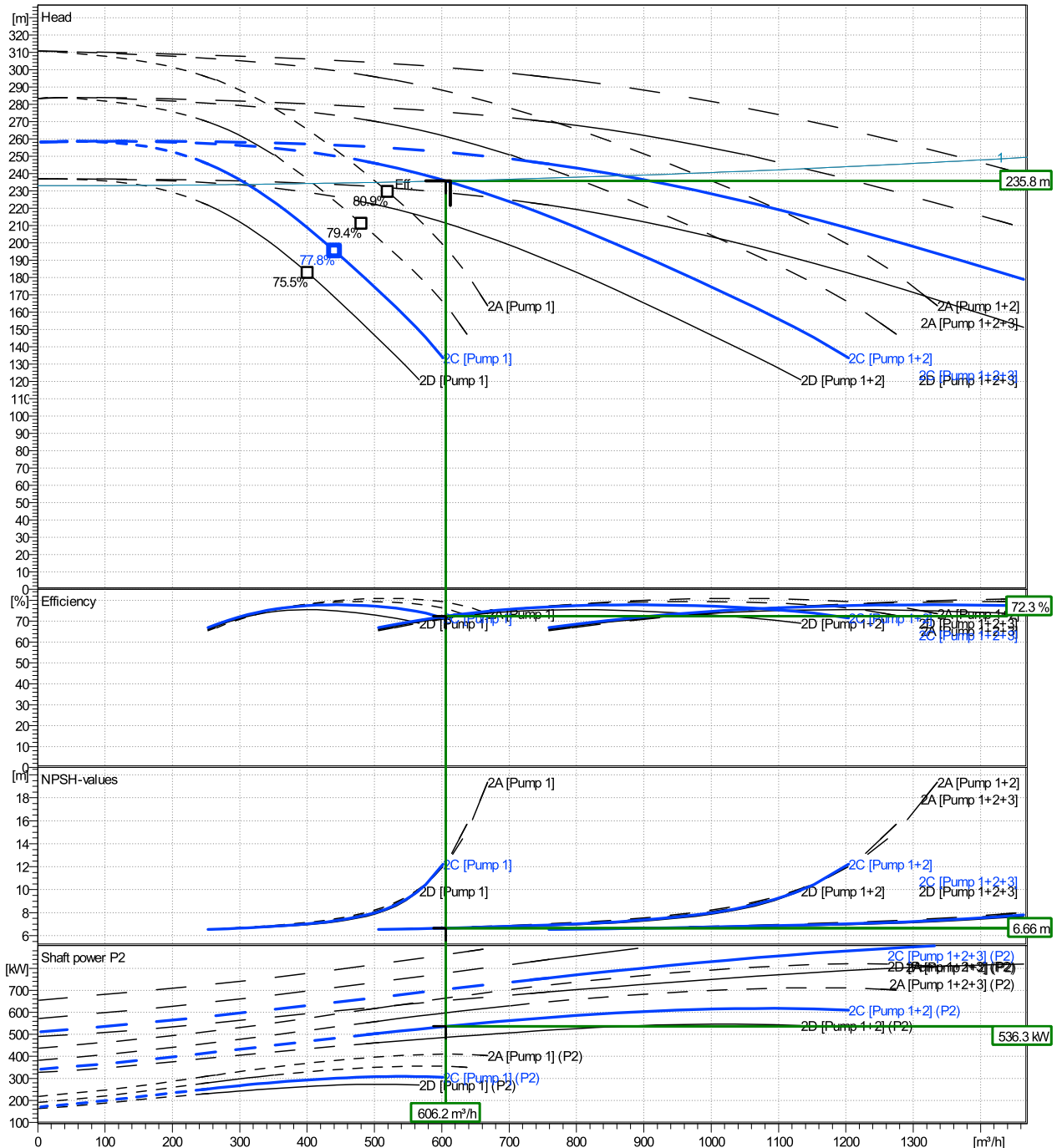
	Ø mm	Pump capacity			Pump head		Shaft power P2			Frequency	
		Min. m³/h	Max. m³/h	Max. m³/h	H(Q=0) m	η Max. m	P2(Q=0) kW	Max. kW	η Max. kW	Hz	
										50	
										2985	
										rpm	2985
										Nominal flow	m³/h
											612.8
actual	304	253	602	441	258	195	618	300		Nominal head	m
Min.	265	/	/	401	237	183	/	263			235.8
Max.	334	/	/	520	311	229	/	400		Inlet pressure	kPa
											0
										Static head	m
											233

Power datas referred to:

hydr. Performance acceptance acc. To EN ISO 9906 Class Grade 2B

Water, pure [100%]; 20°C; 998kg/m³; 1mm²/s

MEI: N.A - according to Ecodesign Directive 2009/125/EC and Regulation (EU) No.547/2012



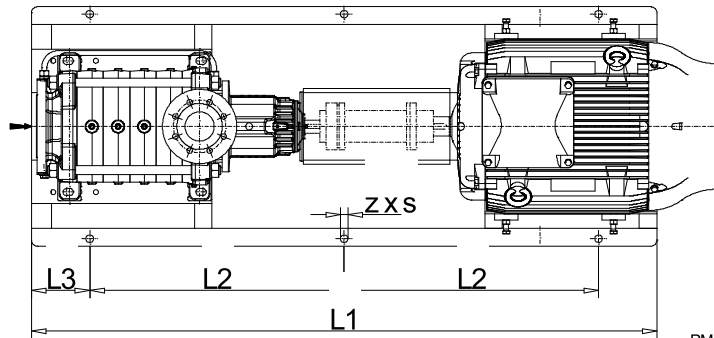
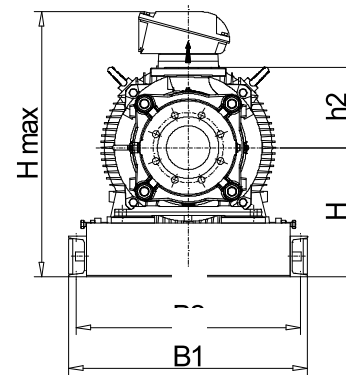
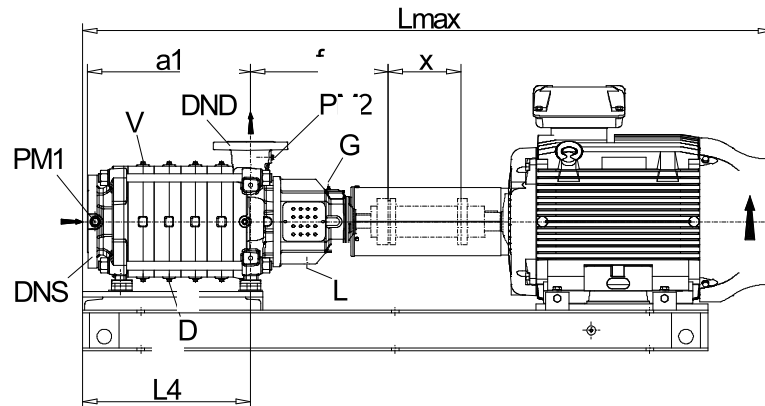
MPA150A/02C/BD3150/L25VDNC4

Dimensions

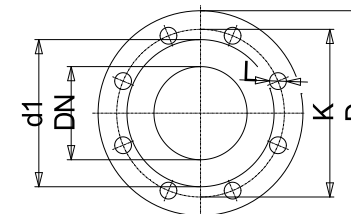
Company name
Contact
Phone number
e-mail address

Complete Unit with Baseplate
AO / DN_s - axial, DN_d - above
3MGS 355 M B3 315 kW

Electrical and dimensional data refer to IE3 motor



PM1...Pressure gauge connector
PM2...Pressure gauge connector
D...Drain
G...Grease nipple
L...Leakage
V...Venting



Note: Value D, C and d may vary from standard

Dimensions [mm]			
a	442	L4	467
B1	1000	Lmax	2632
B2	930	PM1	G1/4
D	G1/4	PM2	G1/4
DN _d	150	s	29
DN _s	250	V	G1/4
f	585	Volumen	2.9189
G	M8	x	250
Guard	525	z	6
H	583		
h2	400		
Hmax	1109		
L	G1/2		
L1	2500		
L2	1000		
L3	250		

Weight (+/- 5%)	
Pump	701
Coupling	32.1
Base plate	361
Motor	1,547
Total weight	2,647 kg

Connections			
Suction nozzle		Discharge nozzle	
DN _s 250		DN _d 150	
PN16		PN25/40	
EN1092-2		EN1092-2	
C	46	C	39
D	450	D	355
d1	319	d1	216
K	355	K	250
L	28	L	28
z	12	z	8

Dimensions and weight without obligation

Project	Xylect-20156882	Created by	roumaissa bouras	Last update	6/11/2024
Block	MPA150A/02C/BD3150/L25VDNC4	Created on	6/11/2024		

MPA125B/02C/BD1600/L25VCCC4

Technical data

Company name
Contact
Phone number
e-mail address

Operating data						
1	Pumpe type	Single pumps as parallel circuit		Fluid	Water, pure	
2	No. of pumps	4		Operating temperature at t A	°C	20
3	Nominal flow	m³/h	612.8	Max / Min Operating Temperature	°C	140 / -25
4	Nominal head	m	199.58	Max / Min Operating Temperature mech. Seal	°C	120 / -10
5	Static head	m	196.8	pH-value at t A	7	
6	Inlet pressure / Vapor pressure at t A	kPa	0 / 100	Density at t A	kg/m³	998
7	Environmental temperature	°C	20	Kin. viscosity at t A	mm²/s	1
8	Available system NPSH	m	0	Altitude	0	

Pump data							
9	Design	Horizontal Multistage pump, axial DN's, radial DN'd, 1 Slide bearing DN's, 1 Roller bearings DN'd					
10	Execution	AO / DN's - axial, DN'd - above					
11	Lubrication	Grease Lubrication					
12	Operating speed	rpm	2985	Stages	2		
13	Flange connections	EN1092-2 / DN's200, PN16 - DN'd125, PN25/40					
14	EN1092-2 / DN's200, PN16 - DN'd125, PN25/40						
15	Max. casing pressure	kPa	2500	Impeller Ø	Max.	mm	295
16	Max. working pressure	kPa	2099.4		designed	mm	2x 270 mm
17	Impeller type	Radial impeller			Min.	mm	235
18	Head H(Q=0)	m	210		Nominal	m³/h	612.8
19	Max. shaft power	kW	578.9	Flow	Max-	m³/h	455.3
20	Pump weight	kg	430		Min-	m³/h	161.9
21	Total weight	kg	1,635.4		Nominal	m	199.6
				Head	at Qmax	m	106
					at Qmin	m	206.6
				Shaft power	kW	459.3	
				Efficiency	%	72.4	
				NPSH 3%	m	4.8	

Materials							
22		Pump		Shaft Seal			
23	Suction Impeller	Cast Iron, EN-GJL-200, ASTM Class 30		Single mechanical seal, with shaft sleeve (unbalanced)			
24	Impeller	Cast Iron, EN-GJL-200, ASTM Class 30		eMG12 - Ø65mm	BQ7EGG-WA		
25	Diffuser	Cast Iron, EN-GJL-150, ASTM Class 25		Mechanical seal diameter	65 mm		
26	Stage Casing	Cast Iron, EN-GJL-250, ASTM Class 35		1. Rotating ring	Carbon graphite resin impregnated		
27	Suction Casing	Cast Iron, EN-GJL-250, ASTM Class 35		2. Stationary ring	SiC, silicon carbide, sintered press. less		
28	Discharge Casing	Cast Iron, EN-GJL-250, ASTM Class 35		3. Secondary seal	Ethylene propylene rubber (EPDM)		
29	Seal Cover	Cast Iron, EN-GJL-250, ASTM Class 35		4. Springs	CrNiMo - Steel		
30	Bearing Bracket / Motor Adapter	Cast Iron, EN-GJL-250, ASTM Class 35		5. Others	EPDM - WRAS		
31	Pump Foot	Carbon Steel, 1.0038, ASTM Grade C, D		Gaskets of the pump	Ethylene propylene rubber (EPDM)		
32	Wear ring	without [STD]					
33	Drum	Stainless Steel, 1.4057, ASTM 431					
34	Drum Bush	Cast Iron, EN-GJL-250, ASTM Class 35					
35	Shaft	Stainless Steel, 1.4057, ASTM 431					
36	Shaft Sleeve	Stainless Steel, 1.4057, ASTM 431					
37	Spacer Sleeve	Stainless Steel, 1.4057, ASTM 431					
38	Shaft Nut	Stainless Steel, 1.4057, ASTM 431					
39	Impeller Nut	A4					
40							
41							
42							
43							

Motor data				Coupling			
44	Manufacturer	Lowara		Manufacturer	FLENDER		
45	Specific design	IE4 3ph Surface Motor - Super Premium Efficiency		Series	Standard Coupling - N-EUPEX - Type B		
46	Type	3MGS 315 M B3 160 kW E4		Shaft diameter	Pump / Motor	mm	52.0 / 65.0
47	Rated power	160 kW	Item no.	Frame size	160		
48	Nominal speed	2985 rpm	Service factor	Spacer length	mm	4	
49	Frame size	315 M	Electric data	Weight	kg	7.8	
50	Weight	kg	955.0	Shaft diameter	65.0	mm	
				Coupling protection	e-MP_GR.COUPPLGUARD ES42-230-23 A4 3.4 kg		
Base plate				Remarks			
51	Name	FRAME EMP125-B-312					
52	Weight	kg	239.2				

Project	Xylect-20156918	Created by	roumaissa bouras	Last update	6/11/2024
Block	MPA125B/02C/BD1600/L25VCCC4	Created on	6/11/2024		

MPA125B/02C/BD1600/L25VCCC4

Performance curve

Company name
Contact
Phone number
e-mail address

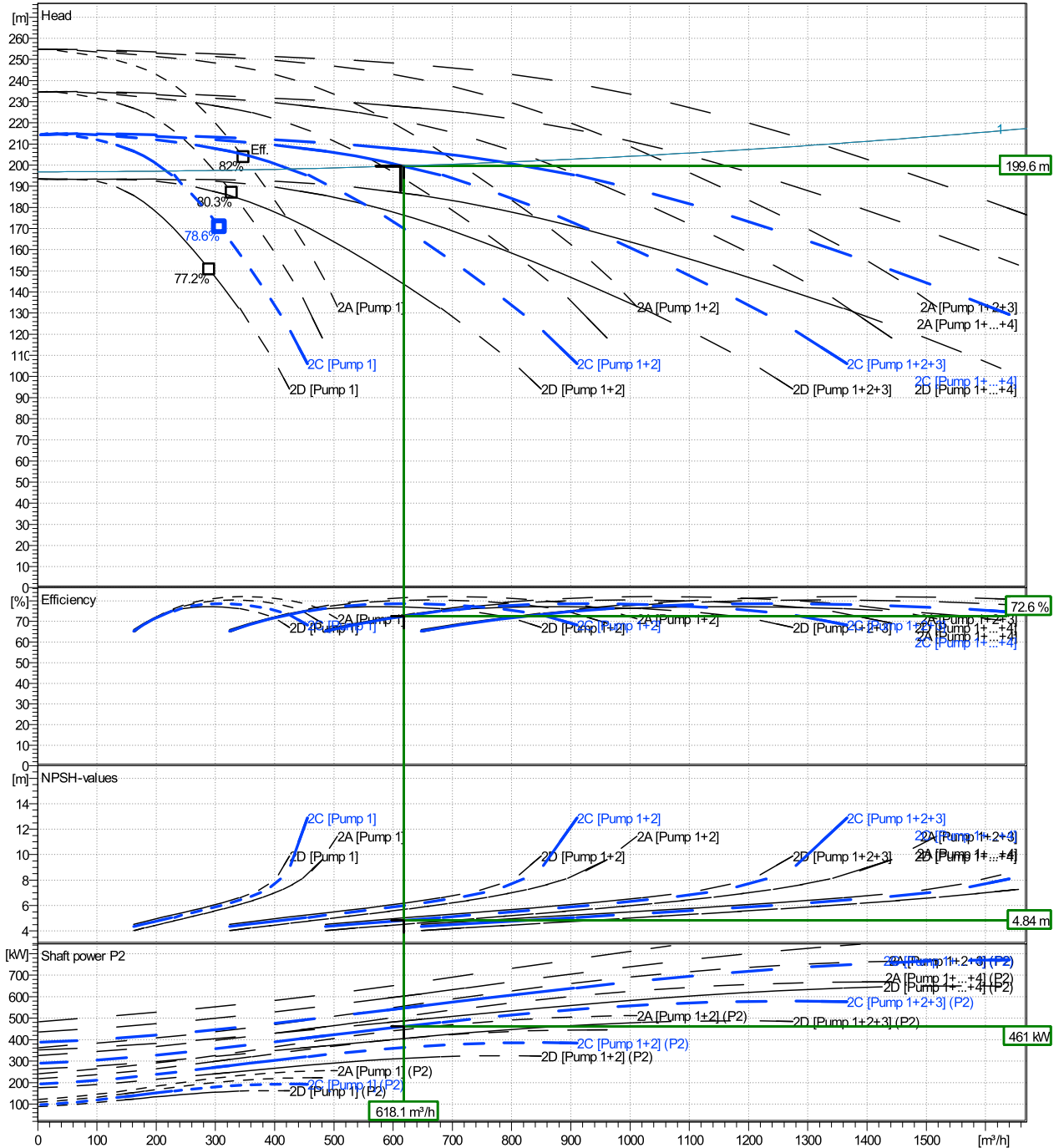
	Ø mm	Pump capacity			Pump head		Shaft power P2			Frequency	Hz	
		Operating range Min. m³/h	Max. m³/h	η Max. m³/h	H(Q=0) m	η Max. m	P2(Q=0) kW	Max. kW	η Max. kW	Operating speed	rpm	
actual	270	177	226	307	214	171	579	181	181	Nominal flow	m³/h	612.8
Min.	235	/	/	290	193	150	/	/	153	Nominal head	m	199.6
Max.	295	/	/	348	255	204	/	/	234	Inlet pressure	kPa	0
										Static head	m	196.8

Power datas referred to:

hydr. Performance acceptance acc. To EN ISO 9906 Class Grade B2B

Water, pure [100%]; 20°C; 998kg/m³; 1mm²/s

MEI: N.A - according to Ecodesign Directive 2009/125/EC and Regulation (EU) No.547/2012



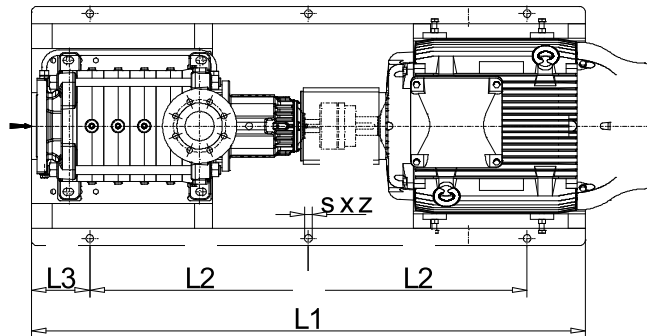
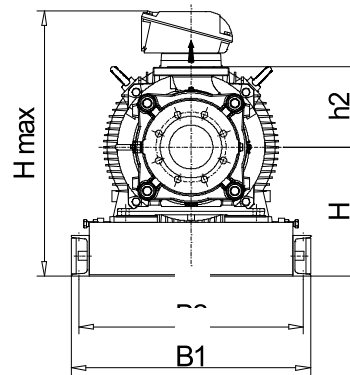
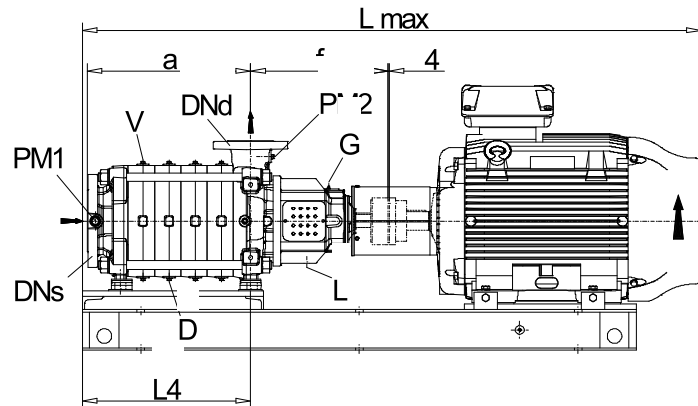
MPA125B/02C/BD1600/L25VCCC4

Dimensions

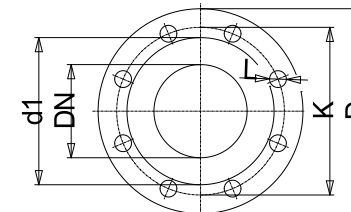
Company name
Contact
Phone number
e-mail address

Complete Unit with Baseplate
AO / DN_s - axial, DN_d - above
3MGS 315 M B3 160 kW E4

Electrical and dimensional data refer to IE4 motor



PM1...Pressure gauge connector
PM2...Pressure gauge connector
D...Drain
G...Grease nipple
L...Leakage
V...Venting



Note: Value D, C and d may vary from standard

Dimensions [mm]			
a	388	L4	390
B1	860	Lmax	2008
B2	810	PM1	G1/4
D	G1/4	PM2	G1/4
DN _d	125	s	26
DN _s	200	V	G1/4
f	488	Volumen	1.7839
G	M8	z	6
Guard	254		
H	508		
h2	325		
Hmax	1033		
L	G1/2		
L1	1950		
L2	775		
L3	200		

Weight (+/- 5%)	
Pump	430
Coupling	7.8
Base plate	239
Motor	955
Total weight	1,635 kg

Connections			
Suction nozzle		Discharge nozzle	
DN _s 200		DN _d 125	
PN16		PN25/40	
EN1092-2		EN1092-2	
C	40	C	30
D	380	D	280
d1	266	d1	184
K	295	K	220
L	23	L	28
z	12	z	8

Dimensions and weight without obligation

Project	Xylect-20156918	Created by	roumaissa bouras	Last update	6/11/2024
Block	MPA125B/02C/BD1600/L25VCCC4	Created on	6/11/2024		

PA 154/2-SA111-16004

Technical data

Company name
Contact
Phone number
e-mail address

Operating data						
1	Pumpe type	Single pumps as serial circuit		Fluid	Water, pure	
2	No. of pumps	4		Operating temperature t A	°C 20	
3	Nominal flow	m ³ /h	481.7	pH-value at t A	7	
4	Nominal head	m	225.4	Density at t A	kg/m ³ 998	
5	Static head	m	210	Kin. viscosity at t A	mm ² /s 1	
6	Inlet pressure	kPa	0	Vapor pressure at t A	kPa 100	
7	Environmental temperature	°C	20	Content of solid%	Solid size mm 0 0	
8	Available system NPSH	m	0	Altitude	m 0	
Pump data						
9	Pump designation	PA 154/2-SA111-16004		Impeller Ø	Max. mm 370	
10	Design	Horizontal			designed mm 366.6	
11	Execution	LO, DN _s -left/ DND _s -above aggregate (standard)			Min. mm 300	
12	Operating speed	rpm	1490	Flow	Nominal m ³ /h 495.3 (495.3)	
13	Number of stages	2			Max- m ³ /h 696.7	
14	Suction nozzle	DN 200/ PN10 /EN 1092			Min- m ³ /h 101	
15	Discharge nozzle	DN 150/ PN16 /EN 1092		Head	Nominal m 226.3	
16	Max. casing pressure	kPa	3500		at Qmax m 34.3	
17	Max. working pressure	kPa	1089		at Qmin m 107.7	
18	Impeller type	Impeller design	Radial impeller Closed	Shaft power	kW 386.4 (128.8)	
19	Bearing bracket			Max. shaft power	kW 431.5	
20	Lubrication			Efficiency	% 79.01	
21	Constant level Oiler			NPSH 3%	m 6.1	
22	Pump weight / Total weight	kg	/ 2103	Head H(Q=0)	m 110	
Materials						
23	Pump			Shaft Seal		
24	Impeller	Cast iron, 0.6025		mechanical seal		
25	suc.-, disch.-, stage casing	Cast iron, 0.6025		SA = unbalanced, at both sides	BQ1EGG/BQ1EGG	
26	Diffusor	Cast iron, 0.6025		Suction side	.	
27	lantern	Cast iron, 0.6025		1.1 Rotating ring	Carbon, resinimpregnated	
28	Bearing frame	Cast iron, 0.6025		1.2 Stationary ring	Silicon carbide pressureless sintered	
29	shaft, shaft sleeve	alloysteel, 1.4021		1.3 Secondary seal	EPDM	
30	Wear ring	without		1.4 Springs	CrNiMo - steel (1.4571)	
31	Elastics	EPDM		1.5 Others	CrNiMo - steel (1.4571)	
32				Discharge side	.	
33				2.1 Rotating ring	Carbon, resinimpregnated	
34				2.2 Stationary ring	Silicon carbide pressureless sintered	
35				2.3 Secondary seal	EPDM	
36				2.4 Springs	CrNiMo - steel (1.4571)	
37				2.5 Others	CrNiMo - steel (1.4571)	
Motor data				Coupling		
38	Manufacturer	Lowara by Omega	Electric voltage	400 V	Manufacturer	Flender
39	Specific design	IE4 3ph Surface Motor - Super Premium Efficiency		Series	Standard Coupling - N-EUPEX - Type B	
40	Type	3MGS 315 M B3 160 kW E4		Type	B 200/4-65/80	
41	Rated power	160 kW	Electric current	280 A	Frame size	200
42	Nominal speed	1490 rpm	Degree of protection	IP55	Spacer length mm	4
43	Frame size	315 M	Explosion protection	-- -- IEC	Weight kg	16
44	Weight	1068 kg	Shaft diameter	80 mm	Coupling protection	yes Werkstoff :
Base plate				Accessories		
45	Name	DFF		Piping		
46	Weight	kg	270	Heizmantel Gehaeuse	<input type="checkbox"/> yes / no	
Remarks				Heizmantel Gehaeusedeckel	<input type="checkbox"/> yes / no	
47				Constant level Oiler	<input type="checkbox"/> yes / no	
48				Anchorbolts	<input type="checkbox"/> yes / no	
49					<input type="checkbox"/> yes / no	
50					<input type="checkbox"/> yes / no	

PA 154/2-SA111-16004

Performance curve

Company name
Contact
Phone number
e-mail address

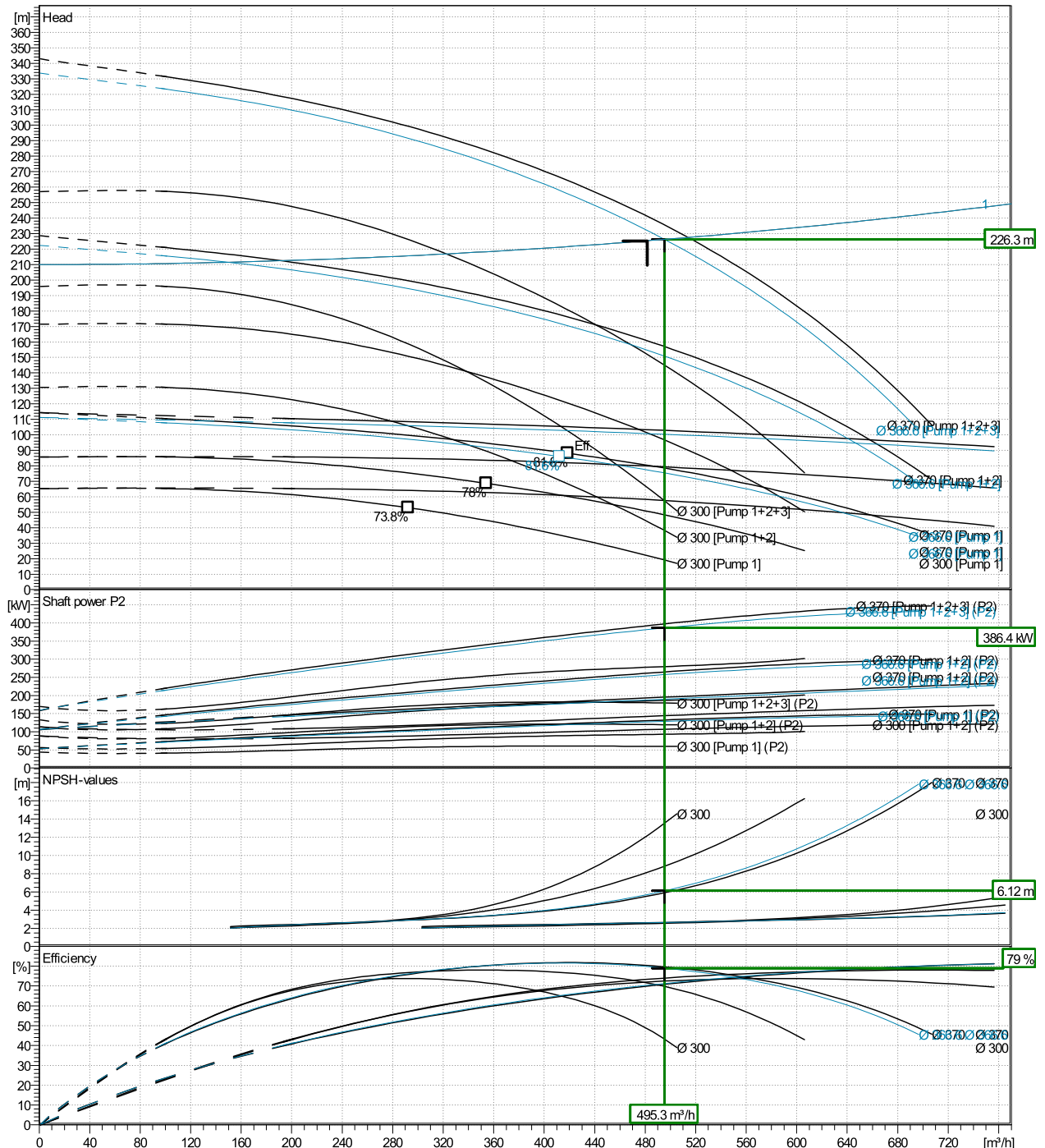
	Ø mm	Pump capacity Operating range			Pump head		Shaft power P2			Impeller type		Radial impeller		
		Min. m³/h	Max. m³/h	η m³/h	H(Q=0) m	η m	P2(Q=0) kW	Max. kW	η kW	Impeller design	Closed			
actual	367	101	697	412	111	86				Sense of rotation	Clockwise from the drive end			
Min.	300	/	/	292	65.3	52.9				Discharge width	mm	35.0		
Max.	370	/	/	419	114	88.1				Free passage	mm			
											Frequency	Hz	50	
										Duty chart	1450/2	Operating speed	rpm	1490

Power datas referred to:

Water, pure [100%]; 20°C; 998kg/m³; 1mm²/s

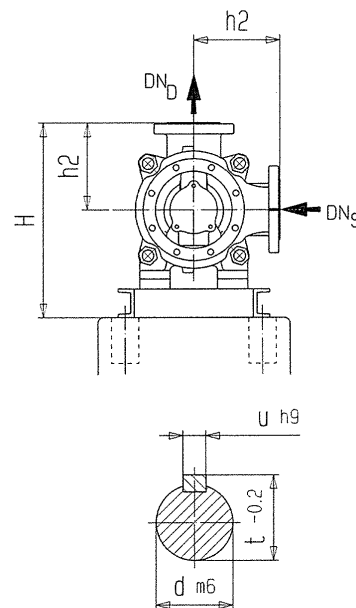
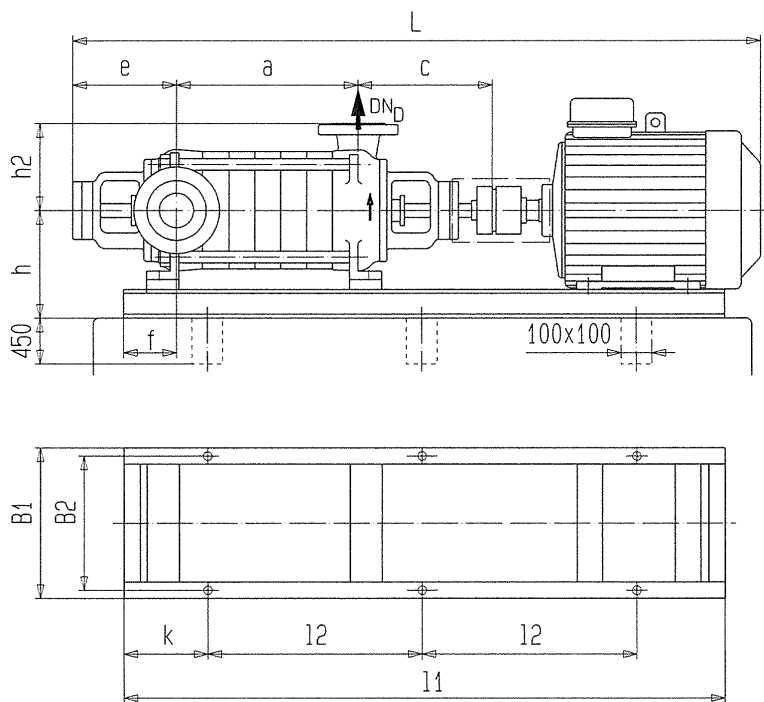
hydr. Leistungsprüfung nach EN ISO 9906 Klasse

Grade 2B



Unit with standard motor brand and accessories

LO, DN_s-left/ DN_d-above aggregate (standard)



Dimensions [mm]

a	425
B1	900
B2	850
c	565
d	65
e	440
f	300
h	518
H	918
h2	400
k	200
L	2586
l1	2400
l2	1000
t	69
u	18
Volumen	2.1366

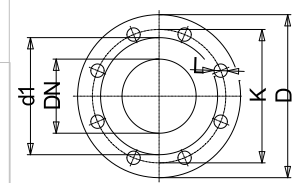
Dimensions and weight without obligation

Connections [mm]

Suction nozzle	Discharge nozzle
DN 200	DN 150
PN10	PN16
EN 1092	EN 1092

Weight	[kg]
Pump	16
Coupling	270
Base plate	1068
Motor	~ 2103

C	20	C	24
d	268	d	212
D	340	D	285
K	295	K	240
L	23	L	22
n	8	n	8



MPA65B/04A/BD750/L25VCCC4

Technical data

Company name
Contact
Phone number
e-mail address

Operating data					
1	Pumpe type	Single pumps as parallel circuit		Fluid	Water, pure
2	No. of pumps	5		Operating temperature t A	°C 20
3	Nominal flow	m³/h	434.5	Max / Min Operating Temperature	°C 140 / -25
4	Nominal head	m	189.93	Max / Min Operating Temperature mech. Seal	°C 120 / -10
5	Static head	m	175	pH-value at t A	7
6	Inlet pressure / Vapor pressure at t A	kPa	0 / 100	Density at t A	kg/m³ 998
7	Environmental temperature	°C	20	Kin. viscosity at t A	mm²/s 1
8	Available system NPSH	m	0	Altitude	0
Pump data					
9	Design	Horizontal Multistage pump, axial DN, radial DN, 1 Slide bearing DN, 1 Roller bearings DN			
10	Execution	AO / DN - axial, DN - above			
11	Lubrication	Grease Lubrication			
12	Operating speed	rpm	2985	Stages	4
13	Flange connections				
14	EN1092-2 / DN125, PN10/16 - DN65, PN25/40				
15	Max. casing pressure	kPa	4000	Impeller Ø	Max. mm 214
16	Max. working pressure	kPa	2476.2		designed mm 4x 214 mm
17	Impeller type	Radial impeller			Min. mm 170
18	Head H(Q=0)	m	250	Flow	Nominal m³/h 431
19	Max. shaft power	kW	288.5		Max- m³/h 151.8
20	Pump weight	kg	189		Min- m³/h 34.4
21	Total weight	kg	970.4	Head	Nominal m 189.7
					at Qmax m 113.6
					at Qmin m 252.7
				Shaft power	kW 279.5
				Efficiency	% 79.4
				NPSH 3%	m 4.1
Materials					
22		Pump		Shaft Seal	
23	Suction Impeller	Cast Iron, EN-GJL-200, ASTM Class 30		Single mechanical seal, with shaft sleeve (unbalanced)	
24	Impeller	Cast Iron, EN-GJL-200, ASTM Class 30		eMG12 - Ø43mm	BQ7EGG-WA
25	Diffuser	Cast Iron, EN-GJL-150, ASTM Class 25		Mechanical seal diameter	43 mm
26	Stage Casing	Ductile Iron, EN-GJS-400-15, ASTM 65-45-12		1. Rotating ring	Carbon graphite resin impregnated
27	Suction Casing	Cast Iron, EN-GJL-250, ASTM Class 35		2. Stationary ring	SiC, silicon carbide, sintered press. less
28	Discharge Casing	Cast Iron, EN-GJL-250, ASTM Class 35		3. Secondary seal	Ethylene propylene rubber (EPDM)
29	Seal Cover	Cast Iron, EN-GJL-250, ASTM Class 35		4. Springs	CrNiMo - Steel
30	Bearing Bracket / Motor Adapter	Cast Iron, EN-GJL-250, ASTM Class 35		5. Others	EPDM - WRAS
31	Pump Foot	Cast Iron, EN-GJL-250, ASTM Class 35		Gaskets of the pump	Ethylene propylene rubber (EPDM)
32	Wear ring	Duplex Stainless Steel, 1.4462, ASTM F51			
33	Drum	Stainless Steel, 1.4057, ASTM 431			
34	Drum Bush	Cast Iron, EN-GJL-250, ASTM Class 35			
35	Shaft	Stainless Steel, 1.4057, ASTM 431			
36	Shaft Sleeve	Stainless Steel, 1.4057, ASTM 431			
37	Spacer Sleeve	Stainless Steel, 1.4057, ASTM 431			
38	Shaft Nut	Stainless Steel, 1.4057, ASTM 431			
39	Impeller Nut	A4			
40					
41					
42					
43					
Motor data					
44	Manufacturer	Omega		Electrical and dimensional data refer to IE4 motor	
45	Specific design	IE4 3ph Surface Motor - Super Premium Efficiency		Coupling	
46	Type	3MGS 280 S B3 75 kW E4		Manufacturer	FLENDER
47	Rated power	75 kW	Item no.	Series	Standard Coupling - N-EUPEX - Type B
48	Nominal speed	2985 rpm	Service factor	1	
49	Frame size	280 S	Electric data	400 V 129 A	
50	Weight	kg 596.0	Shaft diameter	65.0 mm	
				Shaft diameter	Pump / Motor mm 35.0 / 65.0
				Frame size	160
				Spacer length	mm 4
				Weight	kg 7.8
				Coupling protection	e-MP_GR.COUPPLGUARD ES32-190-23 A4 2.4 kg
Base plate					
51	Name	FRAME EMP65-DE-280			
52	Weight	kg	175.2		
			Remarks		

MPA65B/04A/BD750/L25VCCC4

Performance curve

Company name
Contact
Phone number
e-mail address

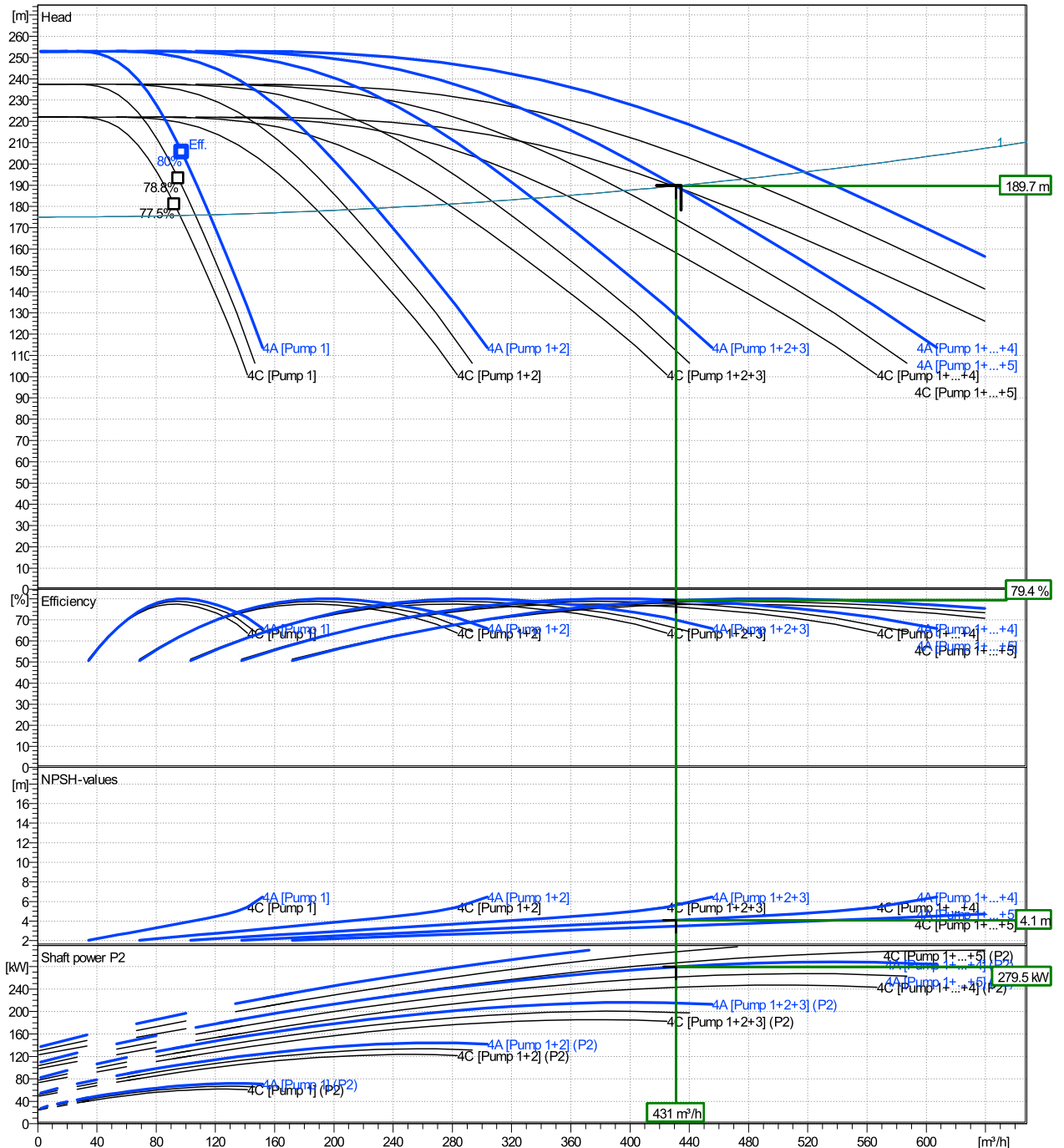
	Ø mm	Pump capacity			Pump head		Shaft power P2			Frequency	Hz	
		Operating range Min. m³/h	Max. m³/h	η Max. m³/h	H(Q=0) m	η Max. m	P2(Q=0) kW	Max. kW	η Max. kW	Operating speed rpm	2985	
actual	214	48.1	115	97.4	253	206	289	67.9	67.9	Nominal flow	m³/h	434.5
Min.	170	/	/	92.4	222	181	/	/	58.6	Nominal head	m	189.9
Max.	214	/	/	97.4	253	206	/	/	67.9	Inlet pressure	kPa	0
										Static head	m	175

Power datas referred to:

hydr. Performance acceptance acc. To EN ISO 9906 Class Grade 2B

Water, pure [100%]; 20°C; 998kg/m³; 1mm²/s

MEI: N.A - according to Ecodesign Directive 2009/125/EC and Regulation (EU) No.547/2012



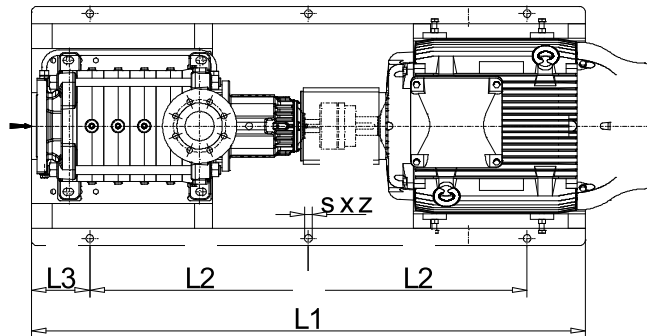
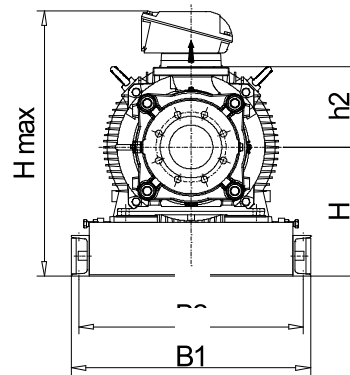
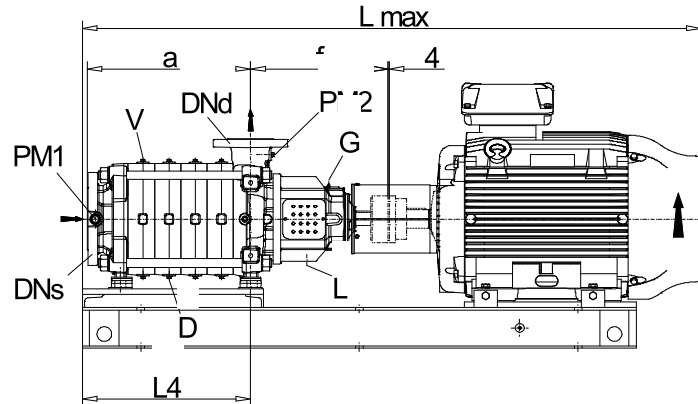
MPA65B/04A/BD750/L25VCCC4

Dimensions

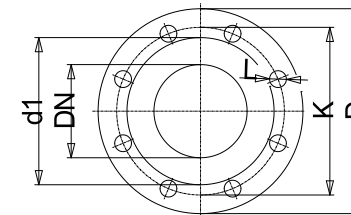
Company name
Contact
Phone number
e-mail address

Complete Unit with Baseplate
AO / DN_s - axial, DN_d - above
3MGS 280 S B3 75 kW E4

Electrical and dimensional data refer to IE4 motor



PM1...Pressure gauge connector
PM2...Pressure gauge connector
D...Drain
G...Grease nipple
L...Leakage
V...Venting



Note: Value D, C and d may vary from standard

Dimensions [mm]			
a	404	L4	487
B1	820	Lmax	1840.5
B2	770	PM1	G1/4
D	G1/4	PM2	G1/4
DN _d	65	s	26
DN _s	125	V	G1/4
f	393	Volumen	1.2919
G	M8	z	6
Guard	224		
H	443		
h2	225		
Hmax	856		
L	G1/2		
L1	1700		
L2	650		
L3	200		

Weight (+/- 5%)	
Pump	189
Coupling	7.8
Base plate	175
Motor	596
Total weight	970 kg

Connections			
Suction nozzle		Discharge nozzle	
DN _s 125		DN _d 65	
PN10/16		PN25/40	
EN1092-2		EN1092-2	
C	30	C	24
D	270	D	190
d1	184	d1	118
K	210	K	145
L	19	L	19
z	8	z	8

Dimensions and weight without obligation

Project	Xylect-20156968	Created by	roumaissa bouras	Last update	6/11/2024
Block	MPA65B/04A/BD750/L25VCCC4	Created on	6/11/2024		